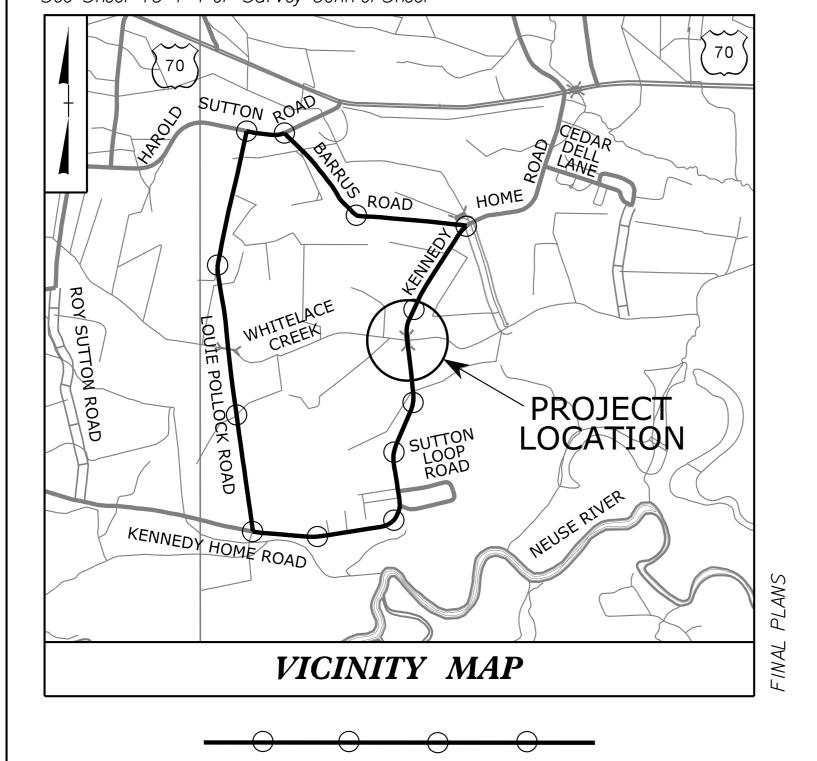
53005. JEC PR

B0041

See Sheet 1A For Index of Sheets See Sheet 1B For Conventional Symbols See Sheet 1C-1 For Survey Control Sheet



OFF SITE DETOUR ROUTE

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

LENOIR COUNTY

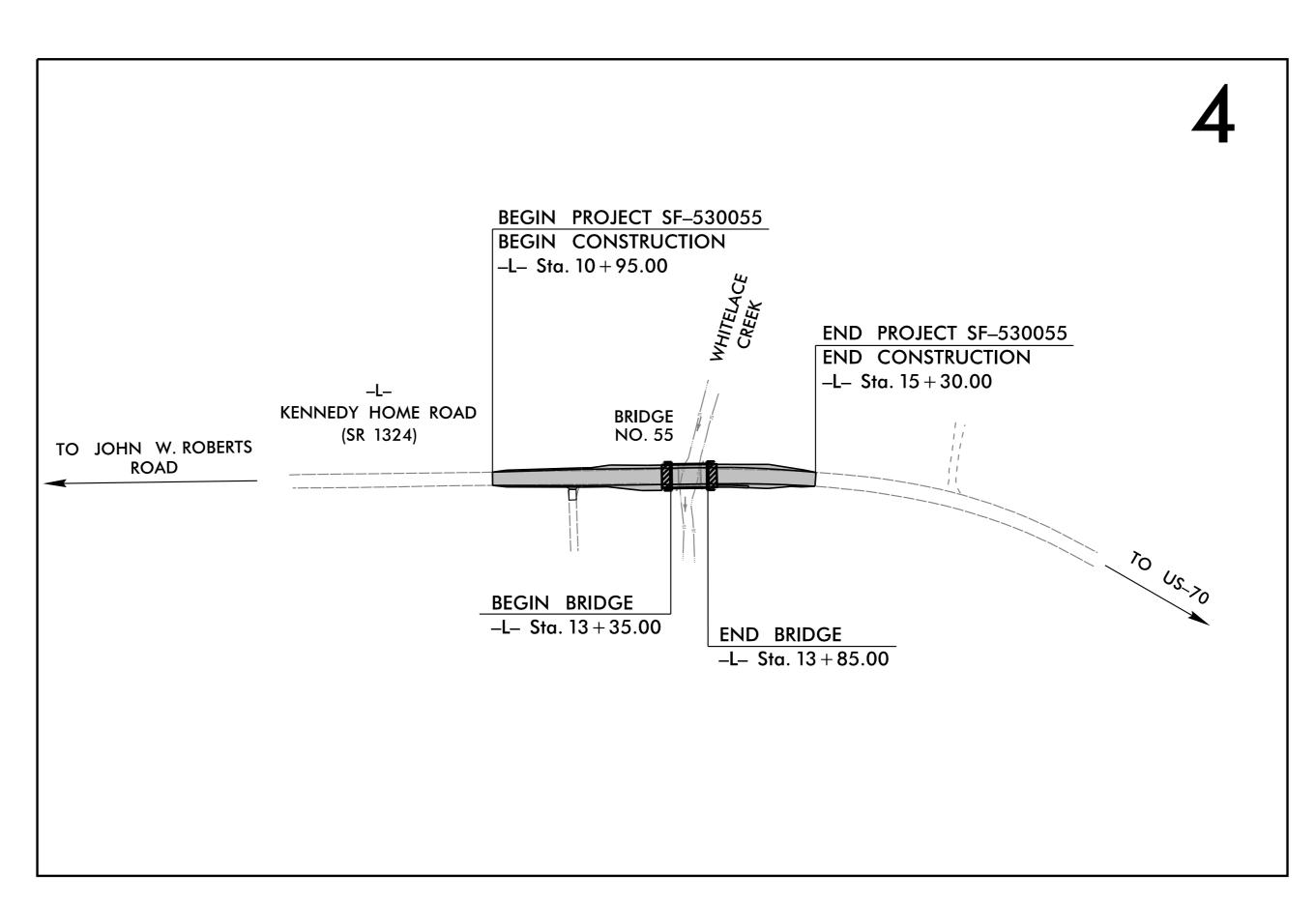
SF-530055 17BP.2.R.78 PE

> DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

NC GRID NAD 83/NA 2011

LOCATION: BRIDGE NO. 55 OVER WHITELACE CREEK ON SR 1324 (KENNEDY HOME ROAD)

TYPE OF WORK: GRADING, DRAINAGE, PAVING, AND STRUCTURE



CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II THIS PROJECT IS NOT WITHIN ANY MUNICIPAL BOUNDARIES.

GRAPHIC SCALES **PLANS** PROFILE (HORIZONTAL) PROFILE (VERTICAL)

DESIGN DATA ADT 2011 = 1700 vpdADT 2040 = 4000 vpd

= 6% 6% 60 MPH

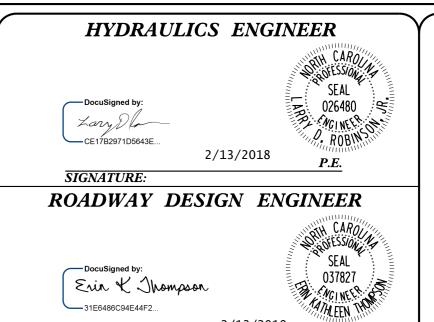
FUNC CLASS = SUB REGIONAL TIER

PROJECT LENGTH

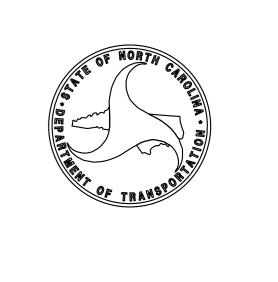
LENGTH ROADWAY PROJECT 17BP.2.R.78 = 0.073 MILES LENGTH STRUCTURE PROJECT 17BP.2.R.78 = 0.009 MILES TOTAL LENGTH PROJECT 17BP.2.R.78 = 0.082 MILES

Kimley » Horn PLANS PREPARED FOR THE NCDOT BY: 2018 STANDARD SPECIFICATIONS ERIN THOMPSON, P.E. PROJECT ENGINEER RIGHT OF WAY DATE: **DECEMBER** 6, 2017

TYLER SPRING, E.I. PROJECT DESIGN ENGINEER LETTING DATE: HEATHER C. LANE, P.E. MARCH 28, 2018 NCDOT CONTACT



SIGNATURE:



STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

Kimley Horn
© 2018

421 FAYETTEVILLE STREET, SUITE 600
RALEIGH, NC 27601

RIGHT-OF-WAY REV.

ROADWAY ENGINEER

CARO

SEAL

037827

Docusioned In NE

31E6486094E444F4+111111

SHEET NO.

PROJECT REFERENCE NO.

SF -530055

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

GENERAL NOTES

2018 SPECIFICATIONS

EFFECTIVE: 01-16-18
REVISED:

GRADE LINE:
GRADING AND SURFACING:

THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. GRADE LINES MAY BE ADJUSTED AT THEIR BEGINNING AND ENDING AND AT STRUCTURES AS DIRECTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

CLEARING:

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II.

SUPERELEVATION:

ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS.

SHOULDER CONSTRUCTION:

ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01

DRIVEWAYS:

DRIVEWAYS SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD. 848.02
USING 3 FOOT RADII OR RADII AS SHOWN ON THE PLANS. LOCATIONS OF DRIVES
WILL BE AS SHOWN ON THE PLANS OR AS DIRECTED BY THE ENGINEER.

GUARDRAIL:

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

SUBSURFACE PLANS:

NO SUBSURFACE PLANS ARE AVAILABLE ON THIS PROJECT. THE CONTRACTOR SHOULD MAKE HIS OWN INVESTIGATION AS TO THE SUBSURFACE CONDITIONS.

END BENTS:

THE ENGINEER SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS-SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROACHING A BRIDGE.

UTILITIES:

UTILITY OWNERS ON THIS PROJECT ARE:

POWER: TRI-COUNTY EMC: TONY GRANTHAM 919-587-9600 TONY.GRANTHAM@TCEMC.COM

WATER/SEWER: NORTH LENOIR WATER CORPORATION: JEFF HARDISON 252-560-1492 JEFFNLWATER@EMARQMAIL.COM

WATER/SEWER: NEUSE REGIONAL WATER & SEWER: HAROLD HERRING 252-522-2567 HAROLD.HERRING@NRWASA.ORG

TELEPONE: CENTURY LINK: ALONZA MITCHELL 252-256-9633 ALONZA.MITCHELL@CENTURYLINK.COM

ANY RELOCATION OF EXISTING UTILITIES WILL BE ACCOMPLISHED BY OTHERS.

PAVEMENT MARKING AND DETOUR:

STATE FORCES WILL INSTALL AND MAINTAIN THE PROJECT DETOUR AND TYPE III BARICADES AT THE PROJECT LIMITS. STATE FORCES WILL INSTALL PAVEMENT MARKINGS AND RAISED PAVEMENT MARKERS ON THE FINAL FINISHED PROJECT. CALL JEFF DUNNING (252-830-3493) WITH FOUR (4) WEEKS' ADVANCED NOTICE FOR COORDINATION.

SF-530055

LENOIR COUNTY

SHEET NUMBER

INDEX OF SHEETS

TITLE SHEET

INDEX OF SHEET

IA INDEX OF SHEETS, GENERAL NOTES, LIST OF ROADWAY STANDARD DRAWINGS
IB CONVENTIONAL SYMBOLS SHEET

IC-I SURVEY CONTROL SHEET

2A-ITHRU 2A-2 PAVEMENT SCHEDULE, TYPICAL SECTIONS, AND MISCELLANEOUS DETAILS

3B-ITHRU 3B-2 SUMMARY SHEETS

3D-I SUMMARY OF DRAINAGE QUANTITIES
4 PLAN SHEET

5 PROFILE SHEET
TMP-ITHRU TMP-2 TRANSPORTATION MANAGEMENT PLANS

SHEET

EC-I THRU EC-6 EROSION CONTROL PLANS
RF-I REFORESTATION PLANS
UO-I THRU UO-2 UTILITIES BY OTHERS PLANS
X-IA CROSS-SECTION SUMMARY SHEET

X-I THRU X-4 CROSS-SECTIONS S-I THRU S-I9 STRUCTURES PLANS EFFECTIVE: 01-16-18 REVISED:

2018 ROADWAY ENGLISH STANDARD DRAWINGS

The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design Branch - N. C. Department of Transportation - Raleigh, N. C., Dated January, 2018 are applicable to this project and by reference hereby are considered a part of these plans:

STD.NO. TITLE

DIVISION 2 - EARTHWORK

200.02 Method of Clearing - Method II

225.02 Guide for Grading Subgrade - Secondary and Local

5.04 Method of Obtaining Superelevation - Two Lane Pavement

DIVISION 3 - PIPE CULVERTS

300.01 Method of Pipe Installation

DIVISION 4 - MAJOR STRUCTURES

422.02 Bridge Approach Fills - Type II Modified Approach Fill

DIVISION 5 - SUBGRADE, BASES AND SHOULDERS

560.01 Method of Shoulder Construction - High Side of Superelevated Curve - Method I

DIVISION 8 - INCIDENTALS

840.00 Concrete Base Pad for Drainage Structures 840.01 Brick Catch Basin - 12" thru 54" Pipe

840.02 Concrete Catch Basin - 12" thru 54" Pipe

840.29 Frames and Narrow Slot Flat Grates

840.46 Traffic Bearing Precast Drainage Structure 840.66 Drainage Structure Steps

846.01 Concrete Curb, Gutter and Curb & Gutter

846.04 Drop Inlet Installation in Shoulder Berm Gutter

848.02 Driveway Turnout - Radius Type

862.01 Guardrail Placement

862.02 GuardrailInstallation

862.03 Structure Anchor Units

876.02 Guide for Rip Rap at Pipe Outlets

K:\RAL_Roadway\011036379 - Bridge55\Roadway\Proj\01

BOUNDARIES AND PROPERTY:

CONVENTIONAL PLAN SHEET SYMBOLS Note: Not to Scale *S.U.E. = Subsurface Utility Engineering

State Line —			
County Line		DAII DOADC.	
Township Line		RAILROADS:	++++++
City Line		Standard Gauge	CSX TRANSPORTATION
Reservation Line	·	RR Signal Milepost	MILE POST 35
Property Line	_ 	Switch —	SWITCH
Existing Iron Pin	_	RR Abandoned	
Property Corner		RR Dismantled	
Property Monument —		RIGHT OF WAY:	
Parcel/Sequence Number		Baseline Control Point —————	•
Existing Fence Line		Existing Right of Way Marker ————	\triangle
Proposed Woven Wire Fence		Existing Right of Way Line	
Proposed Chain Link Fence	- 	Proposed Right of Way Line	
Proposed Barbed Wire Fence	-	Proposed Right of Way Line with Iron Pin and Cap Marker	
Existing Wetland Boundary	wlb	Proposed Right of Way Line with	
Proposed Wetland Boundary		Concrete or Granite RW Marker	-
Existing Endangered Animal Boundary		Proposed Control of Access Line with Concrete C/A Marker	
Existing Endangered Plant Boundary		Existing Control of Access	——————————————————————————————————————
Existing Historic Property Boundary		Proposed Control of Access ————	
Known Contamination Area: Soil	X X	Existing Easement Line —————	——E——
Potential Contamination Area: Soil		Proposed Temporary Construction Easement –	Е
Known Contamination Area: Water	× ×	Proposed Temporary Drainage Easement ——	TDE
Potential Contamination Area: Water ———		Proposed Permanent Drainage Easement —	
Contaminated Site: Known or Potential	- 🕱 🏋	Proposed Permanent Drainage / Utility Easement	
BUILDINGS AND OTHER CULT	URE:	Proposed Permanent Utility Easement ———	
Gas Pump Vent or U/G Tank Cap	- 0	Proposed Temporary Utility Easement ———	
Sign —	_ <u> </u>	Proposed Aerial Utility Easement ———	
Well —	_		AUL
Small Mine	- 🛠	Proposed Permanent Easement with Iron Pin and Cap Marker	♦
Foundation —	-	ROADS AND RELATED FEATURE	
Area Outline	_	Existing Edge of Pavement	
Cemetery	- [†]	Existing Curb	
Building —	_	Proposed Slope Stakes Cut	
School —		Proposed Slope Stakes Fill ————	
Church			_
Dam		Proposed Curb Ramp Existing Metal Guardrail	CR)
HYDROLOGY:		Proposed Guardrail ————————————————————————————————————	
Stream or Body of Water —			
Hydro, Pool or Reservoir —		Existing Cable Guiderail	
Jurisdictional Stream		Proposed Cable Guiderail	_
Buffer Zone 1 ———————————————————————————————————		Equality Symbol ————	•
Buffer Zone 2		Pavement Removal	KXXXXX
Flow Arrow		VEGETATION:	•
Disappearing Stream —————	>	Single Tree	슌
Spring —		Single Shrub	ů
Wetland —		Hedge ————	~~~~~~~
Proposed Lateral, Tail, Head Ditch ————	<u></u>	Woods Line	

Orchard ————————————————————————————————————	& & & &
ineyard ————————————————————————————————————	Vineyard
EXISTING STRUCTURES:	
NAJOR:	
Bridge, Tunnel or Box Culvert ————	CONC
Bridge Wing Wall, Head Wall and End Wall –) CONC WW (
NINOR:	
Head and End Wall ——————————————————————————————————	
ripe Culveri	
Footbridge ————————————————————————————————————	
Drainage Box: Catch Basin, DI or JB ———	СВ
Paved Ditch Gutter———	
Storm Sewer Manhole ————	S
Storm Sewer ————	s
UTILITIES:	
OWER:	
Existing Power Pole ————	•
Proposed Power Pole ————	6
Existing Joint Use Pole ————	-
Proposed Joint Use Pole	-
Power Manhole ——————	P
Power Line Tower ————————————————————————————————————	\boxtimes
Power Transformer ———————————————————————————————————	otag
U/G Power Cable Hand Hole ————	
H-Frame Pole	•—•
U/G Power Line LOS B (S.U.E.*)	P
U/G Power Line LOS C (S.U.E.*)	P
U/G Power Line LOS D (S.U.E.*)	P
ELEPHONE:	
Existing Telephone Pole ————	-•-
Proposed Telephone Pole ————	-0-
Telephone Manhole	①
Telephone Pedestal ————————————————————————————————————	
Telephone Cell Tower ————————————————————————————————————	<u>.</u> .
U/G Telephone Cable Hand Hole ———	√ ▼√
U/G Telephone Cable Hand Hole U/G Telephone Cable LOS B (S.U.E.*) ——	
U/G Telephone Cable LOS C (S.U.E.*)	
U/G Telephone Cable LOS D (S.U.E.*) ——	
U/G Telephone Cable LOS B (S.U.E.*) —	
U/G Telephone Conduit LOS C (S.U.E.*)	
U/G Telephone Conduit LOS D (S.U.E.*)	
U/G Fiber Optics Cable LOS B (S.U.E.*)	
U/G Fiber Optics Cable LOS C (S.U.E.*)	
U/G Fiber Optics Cable LOS D (S.U.E.*)	
5 - 1201 Opiles Cable LOS D (3.0.L.)	

VATER:	
Water Manhole ————	W
Water Meter —————	0
Water Valve —	\otimes
Water Hydrant —	⋄
U/G Water Line LOS B (S.U.E*)	
U/G Water Line LOS C (S.U.E*)	
U/G Water Line LOS D (S.U.E*)	w
Above Ground Water Line —	A/G Water
V :	
TV Pedestal ————————————————————————————————————	
TV Tower —	\otimes
U/G TV Cable Hand Hole ————	HH
U/G TV Cable LOS B (S.U.E.*)	
U/G TV Cable LOS C (S.U.E.*)	
U/G TV Cable LOS D (S.U.E.*)	
U/G Fiber Optic Cable LOS B (S.U.E.*) ——	
U/G Fiber Optic Cable LOS C (S.U.E.*) —	
U/G Fiber Optic Cable LOS D (S.U.E.*)	TV F0
SAS:	
Gas Valve —————	\Diamond
Gas Meter ——————	\Diamond
U/G Gas Line LOS B (S.U.E.*) ————	c
U/G Gas Line LOS C (S.U.E.*)	
U/G Gas Line LOS D (S.U.E.*)	
Above Ground Gas Line	
ANITARY SEWER:	
Sanitary Sewer Manhole	(
Sanitary Sewer Cleanout —————	(
U/G Sanitary Sewer Line —————	·
Above Ground Sanitary Sewer ————	A/G Sanitary Sewer
SS Forced Main Line LOS B (S.U.E.*) ———	
SS Forced Main Line LOS C (S.U.E.*) ———	—— — — FSS— — ——
SS Forced Main Line LOS D (S.U.E.*)——	FSS
AISCELLANEOUS:	
Utility Pole ————————————————————————————————————	•
Utility Pole with Base ————	-
Utility Located Object ————————————————————————————————————	
Utility Traffic Signal Box ———————————————————————————————————	
Utility Unknown U/G Line LOS B (S.U.E.*)	
U/G Tank; Water, Gas, Oil ———————————————————————————————————	
Underground Storage Tank, Approx. Loc. ——	
A/G Tank; Water, Gas, Oil ———————————————————————————————————	
Geoenvironmental Boring ————————————————————————————————————	U
U/G Test Hole LOS A (S.U.E.*) ————————————————————————————————————	
Abandoned According to Utility Records —— End of Information ————————————————————————————————————	
LITO OF ITHORNIUMON ————————————————————————————————————	E.O.I.

False Sump —

DocuSign Envelope ID: 32D96A41-87DD-4A97-A530-DC8E5E90E1E4

SURVEY CONTROL SHEET 53-0055

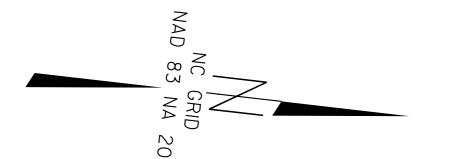
W/EXISTING CENTERLINE ALIGNMENTS PRIOR TO CONSTRUCTION

PROJECT REFERENCE NO. SHEET NO.

17BP.2.R.78 1C-1

Location and Surveys

PROJECT SURVEYOR



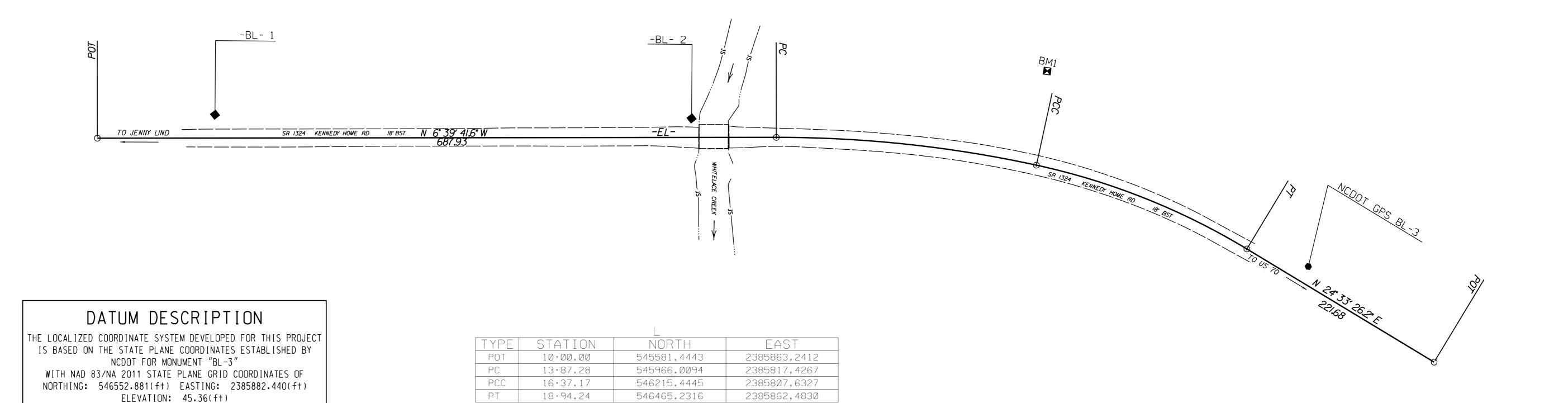
BASELINE

BL					
	POINT	DESC.	NORTH	EAST	ELEVATION
1		BL - 1	545434.7080	2385856.8140	46.87
2		BL - 2	545914.8400	2385805.1510	44.84
3		NCDOT GPS BL-3	546552.8810	2385882.4400	45.36

EL									
POINT	N	E	BEARING	DIST	DELTA	D	L	T	R
POT	545319.067	2385894.173							
LINE			N 06°39′41.6" W	687.93					
PC	546002.355	2385814.369							
CURVE			N 00°29′44.6" W	265.05	12°19′54 . 0"(RT)	04°38′36 . 9"	265.56	133.30	1233.87
PCC	546267.397	2385812.076							
CURVE			N 15°06′49.3" E	229.55	18°53′13 . 9"(RT)	08°11′26.4"	230.59	116.35	699.52
PT	546489.008	2385871.928							
LINE			N 24°33′26.2" E	221.68					
POT	546690.632	2385964.057							

BENCHMARK

BM1 ELEVATION = 45.90 N 546267 E 2385716 R/R SPIKE SET IN POWER POLE



THE AVERAGE COMBINED GRID FACTOR USED ON THIS PROJECT (GROUND TO GRID) IS: 0.99987606

THE N.C. LAMBERT GRID BEARING AND LOCALIZED HORIZONTAL GROUND DISTANCE FROM

"BL-3" TO -L- STATION 10+95.00 IS S 1° 59' 14.9" W 877.63'

ALL LINEAR DIMENSIONS ARE LOCALIZED HORIZONTAL DISTANCES

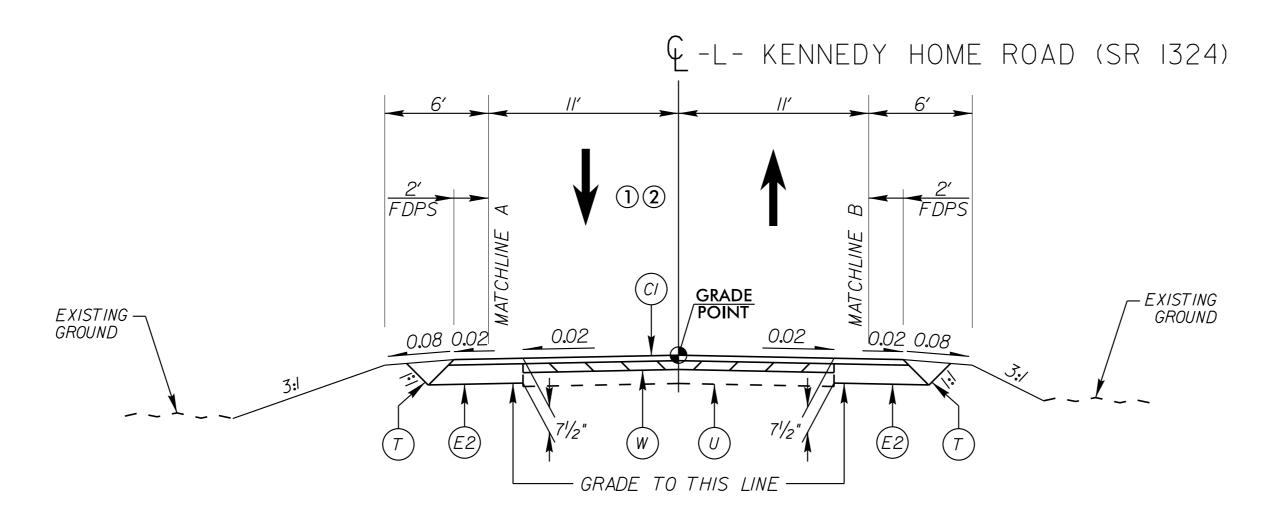
VERTICAL DATUM USED IS NAVD 88

NOTE: DRAWING NOT TO SCALE

I. IF FURTHER INFORMATION REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

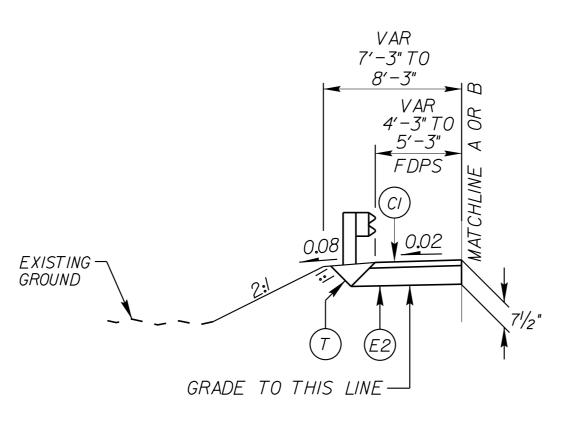
NOTES:

2. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM.



TYPICAL SECTION NO. 1

-L- STA 10+95.00 TO STA 11+70.00 -L- STA 14+40.00 TO STA 15+45.00 **4**)



TYPICAL SECTION NO. 1A

USE IN CONJUNCTION WITH GUARDRAIL LOCATIONS AS FOLLOWS:

-L- STA 14+40.00 TO 14+66.25 (LT & RT)

1) OVERLAY FROM -L- STA 10+95.00 TO STA 11+70.00 AND FROM -L- STA 14+35.00 TO STA 15+30.00 (1.5" S9.5B) 2) MILL NOTCH TO KEY-IN S9.5B FROM -L- STA 10+95.00

2) MILL NOTCH TO KEY-IN S9.5B FROM -L- STA 10+95.00
TO STA 11+20.00 AND -L- STA 15+05.00 TO STA 15+30.00
3) PAVEMENT EDGE SLOPES ARE 1:1 UNLESS SHOWN
OTHERWISE

4) SEE INSETS FOR EXCEPTIONS TO TYPICAL SECTIONS 5) TRANSITION FULL DEPTH SHOULDERS IN AREAS OF 8:1 TAPERS.

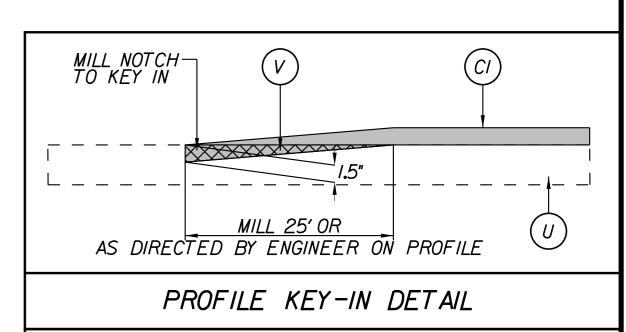
Kimley »

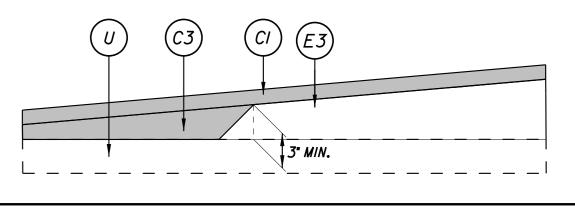
421 FAYETTEVILLE STREET, SUITE 600
RALEIGH, NC 27601

SF -530055		2A-I
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER
CARO OFESSION SEAL 037827 Document of the CINE 31E6486C94E44F2 2/13/2018	Lary	SEAL 026480 PO
2/13/2010		2/13/201

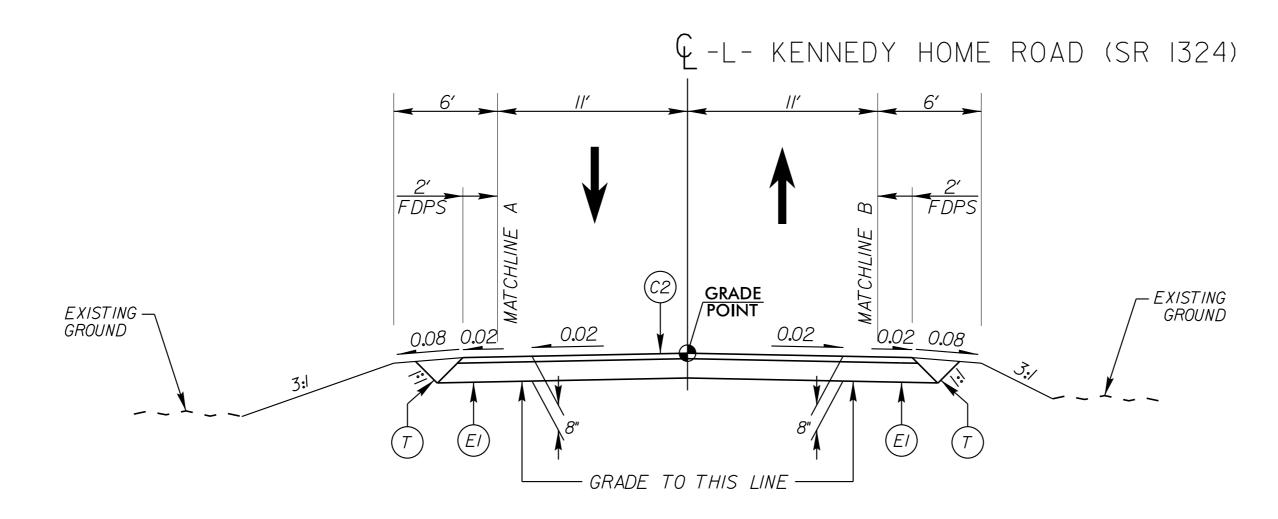
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

	UNLESS ALL SIGNATURES COMPLETED
	FINAL PAVEMENT DESIGN
CI	PROP. APPROX. 1.5" ASPHALT CONCRETE SURFACE COURSE TYPE S9.5B, AT AN AVERAGE RATE OF 168 LBS.PER SQ.YD.
C2	PROP. APPROX. 3" ASPHALT CONCRETE SURFACE COURSE TYPE S9.5B, AT AN AVERAGE RATE OF 168 LBS.PER SQ.YD.IN EACH OF TWO LAYERS
<i>C3</i>	PROP. VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE TYPE S9.5B, AT AN AVERAGE RATE OF 112 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT TO EXCEED 1.5" IN DEPTH
ΕI	PROP. APPROX. 5" ASPHALT CONCRETE BASE COURSE TYPE B25.0C, AT AN AVERAGE RATE OF 570 LBS.PER SQ.YD.
E2	PROP. APPROX. 6" ASPHALT CONCRETE BASE COURSE TYPE B25.0C, AT AN AVERAGE RATE OF 342 LBS.PER SQ.YD.IN EACH OF TWO LAYERS
E3	PROP.VAR.DEPTH ASPHALT CONCRETE BASE COURSE.TYPE B25.OC, AT AN AVERAGE RATE OF 114 LBS.PER SO.YD.PER I" DEPTH.TO BE PLACED IN LAYERS NOT LESS THAN 3" IN DEPTH OR GREATER THAN 5.5" IN DEPTH
R	PROP.SHOULDER BERM GUTTER
T	EARTH MATERIAL
U	EXISTING PAVEMENT
V	MILLING EXISTING PAVEMENT O"TO 1.5" (SEE DETAIL BELOW)
W	VARIABLE DEPTH ASPHALT PAVEMENT (SEE DETAIL BELOW)



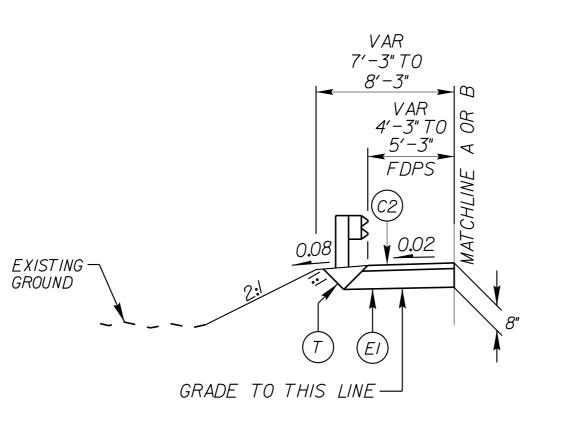


WEDGING DETAIL FOR RESURFACING



TYPICAL SECTION NO. 2

-L- STA ||+70.00 TO STA ||3+35.00 (BEGIN BRIDGE) (4) -L- STA ||3+85.00 (END BRIDGE) TO STA ||4+40.00 (4)



TYPICAL SECTION NO. 2B

-L- STA 13+97.00 TO STA 14+40.00 (RT)

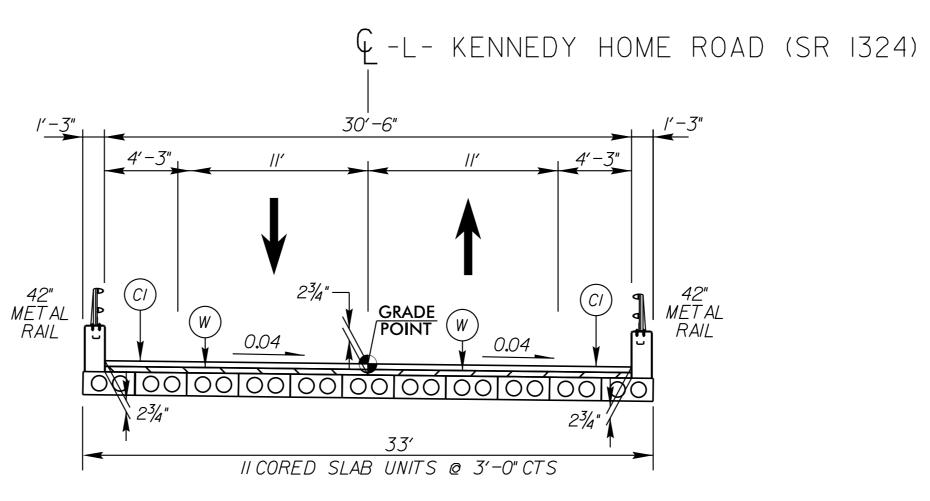
GRADE TO THIS LINE

-EXISTING GROUND

TYPICAL SECTION NO. 2A

USE IN CONJUNCTION WITH GUARDRAIL LOCATIONS AS FOLLOWS:

-L- STA 12+53.75 TO STA 13+35.00 (LT & RT) -L- STA 13+85.00 TO STA 14+40.00 (LT)



BRIDGE TYPICAL SECTION NO. 3

-L- STA 13+35.00 TO STA 13+85.00

Kim

SF -530055		2A-2
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER
SEAL 037827 Docusting feet by CINE 31E6486C94E44F2 2/13/2018	Lan	SEAL 026480 SEAL 026480 POBL 2/13/2018

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

	UNLESS ALL SIGNATURES COMPLETED
	FINAL PAVEMENT DESIGN
CI	PROP. APPROX. 1.5" ASPHALT CONCRETE SURFACE COURSE TYPE S9.5B, AT AN AVERAGE RATE OF 168 LBS.PER SQ.YD.
C2	PROP. APPROX. 3" ASPHALT CONCRETE SURFACE COURSE TYPE S9.5B, AT AN AVERAGE RATE OF 168 LBS.PER SQ.YD.IN EACH OF TWO LAYERS
C3	PROP. VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE TYPE S9.5B, AT AN AVERAGE RATE OF 112 LBS. PER SQ.YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT TO EXCEED 1.5" IN DEPTH
ΕI	PROP. APPROX. 5" ASPHALT CONCRETE BASE COURSE TYPE B25.0C, AT AN AVERAGE RATE OF 570 LBS.PER SQ.YD.
E2	PROP. APPROX. 6" ASPHALT CONCRETE BASE COURSE TYPE B25.0C, AT AN AVERAGE RATE OF 342 LBS.PER SQ.YD.IN EACH OF TWO LAYERS
E3	PROP.VAR.DEPTH ASPHALT CONCRETE BASE COURSE, TYPE B25.0C, AT AN AVERAGE RATE OF 114 LBS.PER SO.YD.PER 1" DEPTH.TO BE PLACED IN LAYERS NOT LESS THAN 3" IN DEPTH OR GREATER THAN 5.5" IN DEPTH
R	PROP.SHOULDER BERM GUTTER
T	EARTH MATERIAL
U	EXISTING PAVEMENT
V	MILLING EXISTING PAVEMENT O"TO 1.5" (SEE DETAIL, SHEET 2A-I)
W	VARIABLE DEPTH ASPHALT PAVEMENT (SEE DETAIL, SHEET 2A-I)

K:\RAL_Roadway\011036379 - B

NOTES:

I) OVERLAY FROM -L- STA 10+95.00 TO STA 11+70.00

AND FROM -L- STA 14+35.00 TO STA 15+30.00 (1.5" S9.5B)

2) MILL NOTCH TO KEY-IN S9.5B FROM -L- STA 10+95.00

TO STA 11+20.00 AND -L- STA 15+05.00 TO STA 15+30.00

3) PAVEMENT EDGE SLOPES ARE I:I UNLESS SHOWN

OTHERWISE

4) SEE INSETS FOR EXCEPTIONS TO TYPICAL SECTIONS

5) TRANSITION FULL DEPTH SHOULDERS IN AREAS OF

/13/2018

8:1 TAPERS.

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 TGS
 DATE:
 11/07/17

 CHECKED BY:
 EKT
 DATE:
 11/07/17

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS SF-530055

SF-530055

A21 FAYETTEVILLE STREET, SUITE 600
RALEIGH, NC 27601

PROJECT REFERENCE NO.

SUMMARY OF EARTHWORK

IN CUBIC YARDS

		EXCAVA	ATION	EMBANKMENT		WASTE
STATION	STATION	TOTAL UNCLASSIFIED	UNDERCUT	EMBANKMENT + 25%	BORROW	TOTAL
-L- 10+95	-L- 13 + 23	3		369	366	
L 13 + 85	_L_ 15+30	30		213	183	
	SUBTOTAL	34		582	549	
	SHOULDER MATERIAL			14	14	
	PROJECT TOTAL	34		596	563	
	PROJECT TOTAL	34		370	363	
EST. 5% TO REPLACE TOPS	OIL ON BORROW PIT				28	
	GRAND TOTAL	34			591	
	SAY	40			600	

NOTE: APPROXIMATE QUANTITIES ONLY. CLEARING AND GRUBBING, UNCLASSIFIED EXCAVATION, BORROW EXCAVATION, FINE GRADING, AND REMOVAL OF EXISTING ASPHALT PAVEMENT WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR "GRADING."

EARTHWORK QUANTITIES ARE CALCULATED BY THE ROADWAY DESIGN UNIT. THESE EARTHWORK QUANTITIES ARE BASED IN PART ON SUBSURFACE DATA PROVIDED BY THE GEOTECHNICAL ENGINEERING UNIT.

N:NAL_RODDWDYNIUJ6J19 - B110yejjnoddwdynF10JNJ

 COMPUTED BY:
 TGS
 DATE:
 11/06/17

 CHECKED BY:
 EKT
 DATE:
 11/06/17

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

PROJECT REFERENCE NO.

SF -530055

SHEET NO.

3B-2

Kimley >> Horn

© 2016

421 FAYETTEVILLE STREET, SUITE 600 RALEIGH, NC 27601

G = GATING IMPACT ATTENUATOR TYPE 350

NG = NON-GATING IMPACT ATTENUATOR TYPE 350

GUARDRAIL SUMMARY

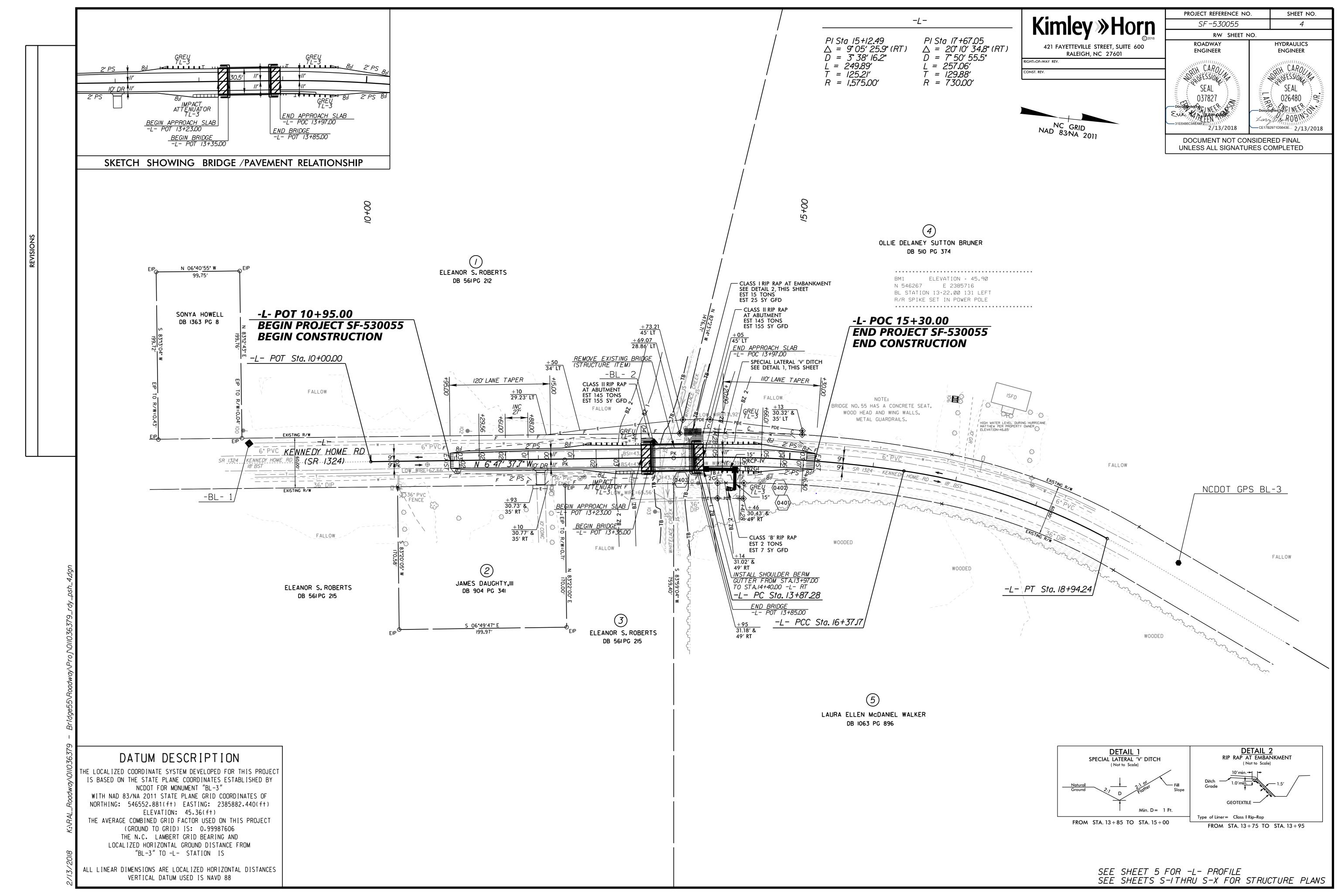
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LINE	BEG. STA.	END STA.	LOCATION	STRAIGHT	SHOP CURVED	DOUBLE FACED	APPROACH END	TRAILING END	GREU TL-3	TEMP GREU TL–3	REU TEMP GREU TL-3	TYPE III	TEMP TYPE III	EA G NG		EA G NG		EA G NG		EA G NG		FA G NG		EA G NG		EA G NG		FA G NG				REMARKS
-L-	12 + 53.75	13 + 35.00	LT	81.25				13+35.00	1		1		\Box																			
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-L-	13 + 85.00	14 + 65.75	LT	81.25			13 + 85.00		1		1																					
-L-	13 + 85.00	14 + 66.75	RT	81.25				13 + 95.00	1		1																					
			SUBTOTAL	243.75																												
	LESS ANCHOR D	DEDUCTIONS																														
	GREU TL-3	3 @ 50′	=	150.00									\vdash																			
	TYPE III	3 @ 18.75′	=	56.25																												
			TOTAL	37.5					3		3		\vdash																			
			SAY	37.5																												

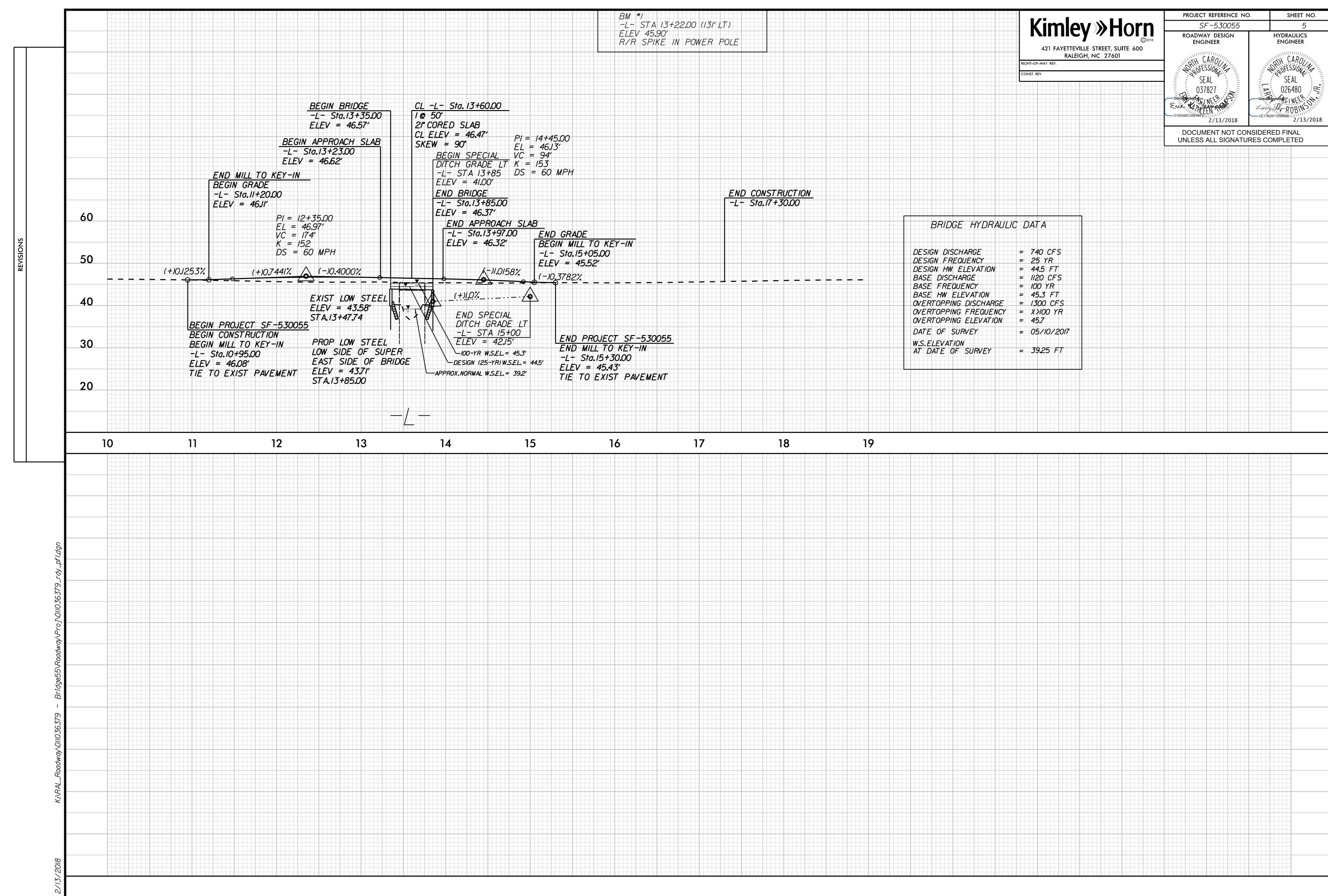
ADDITIONAL GUARDRAIL POSTS = 0 EA

SUMMARY OF SHOULDER BERM GUTTER							
LINE	STATION TO STATION	LOCATION	LENGTH (LF)				
-L-	13+97 TO 14+40	RT	43				
TOTAL			43				
SAY			50				

PROJECT TOTALS

COMPUTED	BY:			НСВ			DATE	≣: 9/21	1/2017																																		PROJECT N	10.	SHEET
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ROADWAY STANDARD DRAWINGS

THE FOLLOWING ROADWAY STANDARDS AS SHOWN IN "ROADWAY STANDARD DRAWINGS" -PROJECT SERVICES UNIT - N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C., DATED JANAUARY 2012 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

STATIONARY WORK ZONE SIGNS

TITLE STD. NO. 1101.03 TEMPORARY ROAD CLOSURES

1145.01 **BARRICADES**

MANAGEMENT STRATEGIES

CONSTRUCTION SUMMARY:

1110.01

PROPOSED BRIDGE REPLACEMENT WILL BE CONSTRUCTED AWAY FROM TRAFFIC USING A ROAD CLOSURE AND DETOUR ROUTE.

GENERAL NOTES

CHANGES MAY BE REQUIRED WHEN PHYSICAL DIMENSIONS IN THE DETAIL DRAWINGS, STANDARD DETAILS AND ROADWAY DETAILS ARE NOT ATTAINABLE TO MEET FIELD CONDITIONS, OR RESULT IN DUPLICATE, OR UNDESIRED OVERLAPPING OF DEVICES. MODIFICATION MAY INCLUDE: MOVING, SUPPLEMENTING, COVERING OR REMOVAL OF DEVICES, AS DIRECTED BY THE ENGINEER.

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT, EXCEPT WHEN OTHERWISE NOTED IN THE PLAN, OR DIRECTED BY THE ENGINEER.

TRAFFIC PATTERN ALTERATIONS

A) NOTIFY THE ENGINEER THIRTY (30) CALENDAR DAYS PRIOR TO ANY TRAFFIC PATTERN ALTERATION.

SIGNING

- B) INSTALL ADVANCE WORK ZONE WARNING SIGNS WHEN WORK IS WITHIN 40 FT FROM THE EDGE OF TRAVEL LANE AND NO MORE THAN THREE (3) DAYS PRIOR TO THE BEGINNING OF CONSTRUCTION.
- C) PROVIDE PERMANENT SIGNING AND DEVICES REQUIRED TO CLOSE THE ROAD ACCORDING TO THE ROADWAY STANDARD DRAWINGS AND TRAFFIC CONTROL PLANS.
- D) PROVIDE SIGNING REQUIRED FOR THE OFF-SITE DETOUR ROUTES AS SHOWN IN THE TRAFFIC CONTROL PLANS.
- E) COVER OR REMOVE ALL SIGNS REQUIRED FOR THE OFF-SITE DETOUR WHEN THE DETOUR IS NOT IN OPERATION.
- F) ENSURE ALL NECESSARY SIGNING IS IN PLACE PRIOR TO ALTERING ANY TRAFFIC PATTERN.

TRAFFIC CONTROL DEVICES

G) PLACE TYPE III BARRICADES, WITH "ROAD CLOSED" SIGN R11-2 ATTACHED, OF SUFFICIENT LENGTH TO CLOSE ENTIRE ROADWAY.

PAVEMENT MARKINGS AND MARKERS

H) TIE PROPOSED PAVEMENT MARKING LINES TO EXISTING PAVEMENT MARKING LINES.

PHASING

- STEP 1: USING ROADWAY STANDARD DRAWING NUMBER 1101.03, SHEET 1 OF 9, AND SHEET TMP-2, PERFORM THE FOLLOWING:
 - INSTALL ALL ROAD CLOSURE AND DETOUR SIGNING, INCLUDING BARRICADES
 - IMPLEMENT A TEMPORARY CLOSURE OF SR 1324 (KENNEDY HOME RD) USING A DETOUR ALONG SR 1335 (LOUIE POLLOCK RD), SR 1330 (HARROLD SUTTON RD), AND SR 1336 (BARRUS RD). ALLOW LOCAL TRAFFIC ACCESS ALONG SR 1324 (KENNEDY HOME RD) TO APPROXIMATELY 0.06 MILES NORTH AND 0.03 MILES SOUTH OF BRIDGE #55 (JUST AFTER THE FINAL DRIVEWAYS PRIOR TO THE BRIDGE.)
- STEP 2: REMOVE EXISTING BRIDGE #55 AND CONSTRUCT THE PROPOSED BRIDGE AND APPROACHES AS SHOWN IN THE CONSTRUCTION PLANS.
- STEP 3: INSTALL ALL FINAL PAVEMENT MARKINGS.
- STEP 4: REMOVE ALL TRAFFIC CONTROL SIGNING AND DEVICES AND OPEN SR 1324 (KENNEDY HOME RD) TO THE FINAL TRAFFIC PATTERN.

NOTE:

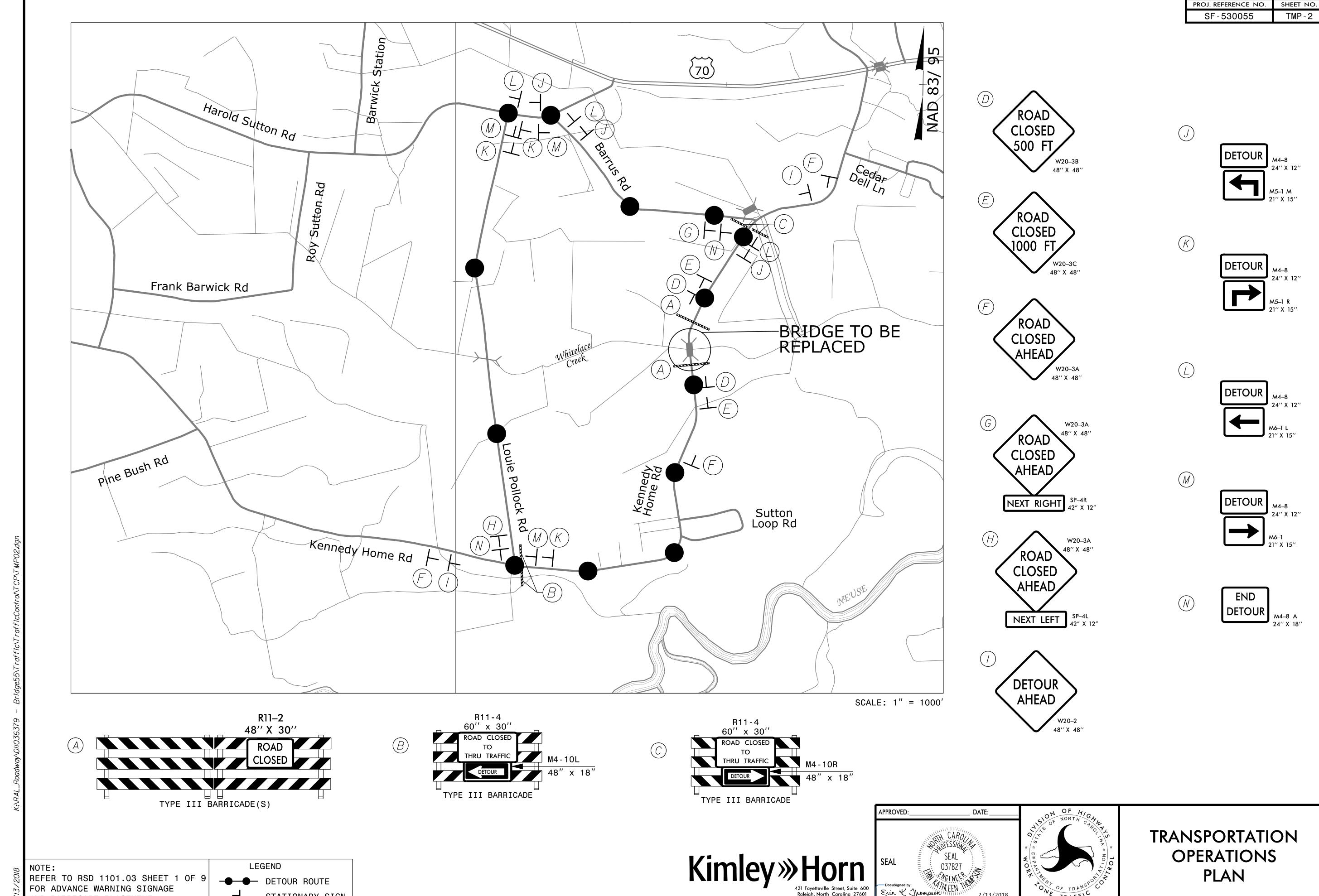
STATE FORCES WILL INSTALL AND MAINTAIN THE PROJECT DETOUR AND TYPE III BARRICADES AT THE PROJECT LIMITS. STATE FORCES WILL REMOVE AND INSTALL PAVEMENT MARKINGS, PERMANENT RAISED PAVEMENT MARKERS, AND SIGNING ON THE FINAL FINISHED PROJECT. CALL JEFF DUNNING (252-830-3493) WITH FOUR (4) WEEKS' ADVANCED NOTICE FOR COORDINATION.







TRANSPORTATION OPERATIONS PLAN



AND BARRICADE PLACEMENT

STATIONARY SIGN

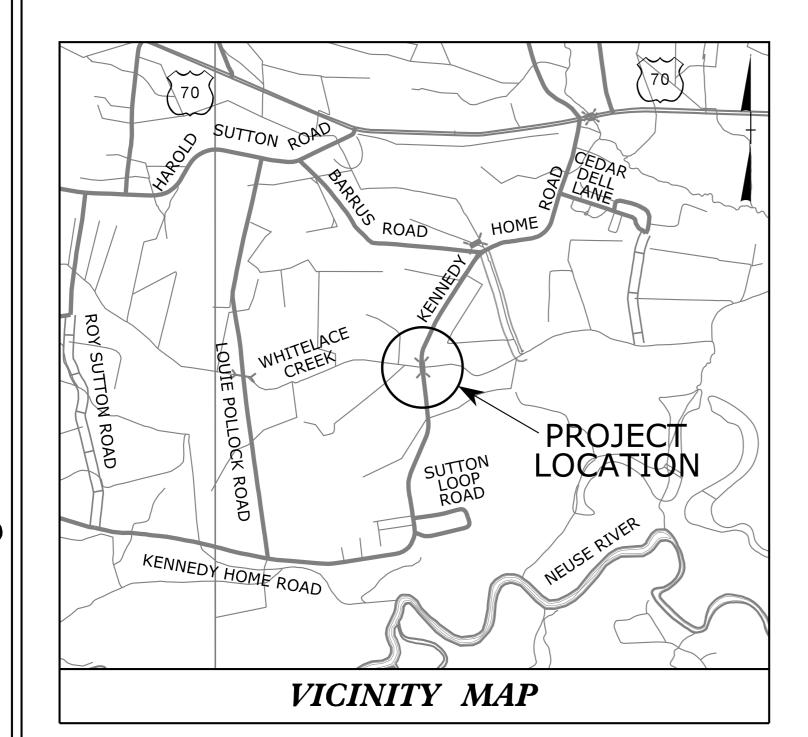
421 Fayetteville Street, Suite 600 Raleigh, North Carolina 27601 C , 2018 PE NO. F–0102





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EROSION AND SEDIMENT CONTROL MEASURES

Description

Temporary Silt Ditch

Temporary Diversion

Temporary Silt Fence

Silt Basin Type B.

Stilling Basin ...

Туре А_

Type B.

Туре С.

Infiltration Basin

Tiered Skimmer Basin...

Skimmer Basin

1630.06 Special Stilling Basin.

1630.04

1632.01

1632.02

1632.03

Special Sediment Control Fence

1633.02 Temporary Rock Silt Check Type-B.

Wattle / Coir Fiber Wattle...

Wattle / Coir Fiber Wattle with Polyacrylamide (PAM)

Rock Inlet Sediment Trap:

Temporary Berms and Slope Drains

Temporary Rock Silt Check Type-A

Temporary Rock Silt Check Type-A with Matting and Polyacrylamide (PAM)

Temporary Rock Sediment Dam Type-A...

Temporary Rock Sediment Dam Type-B....

Rock Pipe Inlet Sediment Trap Type-A....

Rock Pipe Inlet Sediment Trap Type-B.....

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

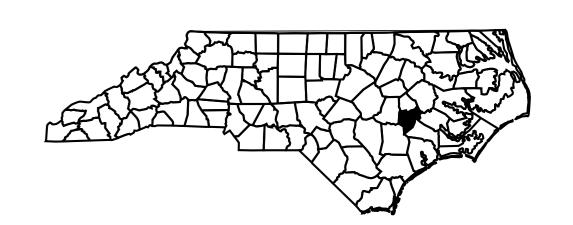
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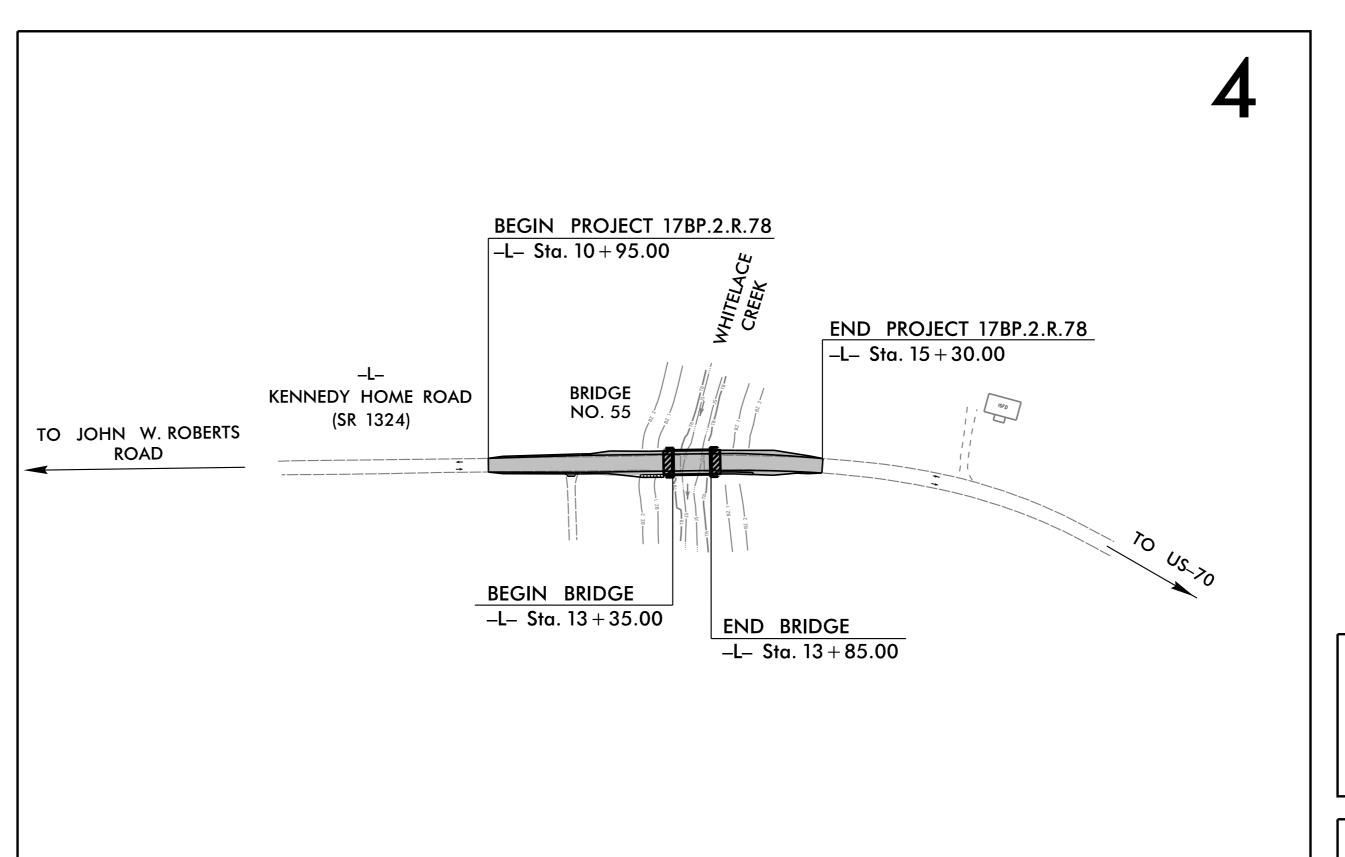
LOCATION: BRIDGE NO. 55 OVER WHITELACE CREEK ON SR 1324 (KENNEDY HOME ROAD)

TYPE OF WORK: GRADING, DRAINAGE, PAVING, AND STRUCTURE

STATE	STATE I	PROJECT REFERENCE NO.	NO.	SHEETS
N.C.	SF	-530055	EC-1	
STAT	E PROJ. NO.	F. A. PROJ. NO.	DESCRIPT	ION
17BF	P.2.R.78			

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED







THIS PROJECT CONTAINS EROSION CONTROL PLANS FOR CLEARING AND GRUBBING PHASE OF CONSTRUCTION.

THIS PROJECT HAS BEEN DESIGNED TO SENSITIVE WATERSHED STANDARDS.

ENVIRONMENTALLY SENSITIVE AREA(S) EXIST ON THIS PROJECT

Refer To E. C. Special Provisions for Special Considerations.

ROADSIDE ENVIRONMENTAL UNIT DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS

2018 STANDARD SPECIFICATIONS

Erin Thompson

LEVEL IIIA NAME

4031 LEVEL IIIA CERTIFICATION NO.

GRAPHIC SCALES PLANS PROFILE (HORIZONTAL) PROFILE (VERTICAL)

ROADSIDE ENVIRONMENTAL UNIT DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

> THESE EROSION AND SEDIMENT CONTROL PLANS COMPLY WITH THE REGULATIONS SET FORTH BY THE NCG-010000 GENERAL CONSTRUCTION PERMIT EFFECTIVE AUGUST 1, 2016 ISSUED BY THE NORTH CAROLINA DEPARTMENT OF ENVIRONMENT AND NATURAL RESOURCES DIVISION OF WATER QUALITY.

PLANS PREPARED FOR THE NCDOT BY: 2018 STANDARD SPECIFICATIONS

LETTING DATE:

FEBRUARY 28, 2018

RIGHT OF WAY DATE: **DECEMBER** 6, 2017 TYLER SPRING, E.I. PROJECT DESIGN ENGINEER

Roadway Standard Drawings

1630.06 Special Stilling Basin

1631.01 Matting Installation

The following roadway english standards as appear in "Roadway Standard Drawings"- Roadway Design Unit - N. C. Department of Transportation - Raleigh, N. C., dated January 2018 and the latest revison thereto are applicable to this project and by reference hereby are considered a part of

tnese pia	пѕ.
1604.01	Railroad Erosion Control Detail
1605.01	Temporary Silt Fence
1606.01	Special Sediment Control Fence
1607.01	Gravel Construction Entrance
1622.01	Temporary Berms and Slope Drains
1630.01	Riser Basin
1630.02	Silt Basin Type B
1630.03	Temporary Silt Ditch

1633.01 Temporary Rock Silt Check Type A 1630.04 Stilling Basin for Pumped Effluent 1630.05 Temporary Diversion

1633.02 Temporary Rock Silt Check Type B 1634.01 Temporary Rock Sediment Dam Type A 1634.02 Temporary Rock Sediment Dam Type B 1635.01 Rock Pipe Inlet Sediment Trap Type A 1635.02 Rock Pipe Inlet Sediment Trap Type B 1640.01 Coir Fiber Baffle

1645.01 Temporary Stream Crossing

1632.01 Rock Inlet Sediment Trap Type A

1632.02 Rock Inlet Sediment Trap Type B

1632.03 Rock Inlet Sediment Trap Type C

Kimley » Horn

ERIN THOMPSON, P.E. PROJECT ENGINEER

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STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

JECT REFERENCE NO. SHEET NO SF - 530055 EC - 2

SOIL STABILIZATION TIMEFRAMES

SITE DESCRIPTION	STABILIZATION TIME	TIMEFRAME EXCEPTIONS
PERIMETER DIKES, SWALES, DITCHES AND SLOPES	7 DAYS	NONE
HIGH QUALITY WATER (HQW) ZONES	7 DAYS	NONE
SLOPES STEEPER THAN 3:1	7 DAYS	IF SLOPES ARE 10'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1,14 DAYS ARE ALLOWED.
SLOPES 3:1 OR FLATTER	I4 DAYS	7 DAYS FOR SLOPES GREATER THAN 50'IN LENGTH.
ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1	I4 DAYS	NONE, EXCEPT FOR PERIMETERS AND HOW ZONES.

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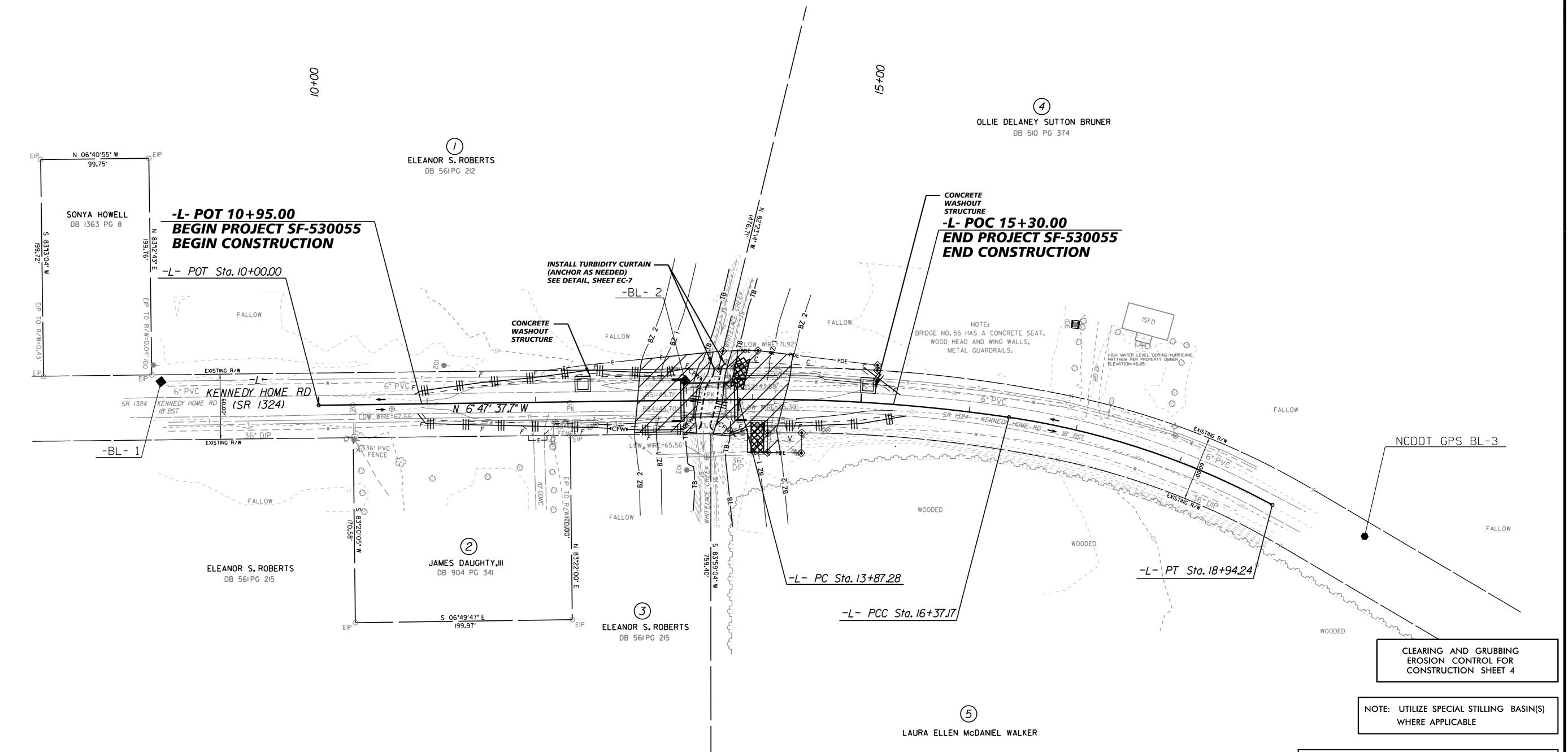
/13/2018

EROSION CONTROL PLAN

PROJECT REFERENCE NO. SHEET NO. SF -530055 EC-3 R/W SHEET NO. CONST. 4

421 FAYETTEVILLE STREET, SUITE 600 RALEIGH, NC 27601





DATUM DESCRIPTION

THE LOCALIZED COORDINATE SYSTEM DEVELOPED FOR THIS PROJECT IS BASED ON THE STATE PLANE COORDINATES ESTABLISHED BY NCDOT FOR MONUMENT "BL-3"

WITH NAD 83/NA 2011 STATE PLANE GRID COORDINATES OF NORTHING: 546552.881(ft) EASTING: 2385882.440(ft) ELEVATION: 45.36(ft)

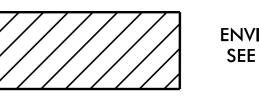
THE AVERAGE COMBINED GRID FACTOR USED ON THIS PROJECT (GROUND TO GRID) IS: 0.99987606 THE N.C. LAMBERT GRID BEARING AND LOCALIZED HORIZONTAL GROUND DISTANCE FROM

ALL LINEAR DIMENSIONS ARE LOCALIZED HORIZONTAL DISTANCES

"BL-3" TO -L- STATION IS

NOTES: ANY DEVIATION FROM OPTIONS GIVEN WILL REQUIRE PRIOR APPROVAL BY ENGINEER.

ADDITIONAL EROSION CONTROL DEVICES MAY NEED TO BE INSTALLED AS DIRECTED BY THE ENGINEER.



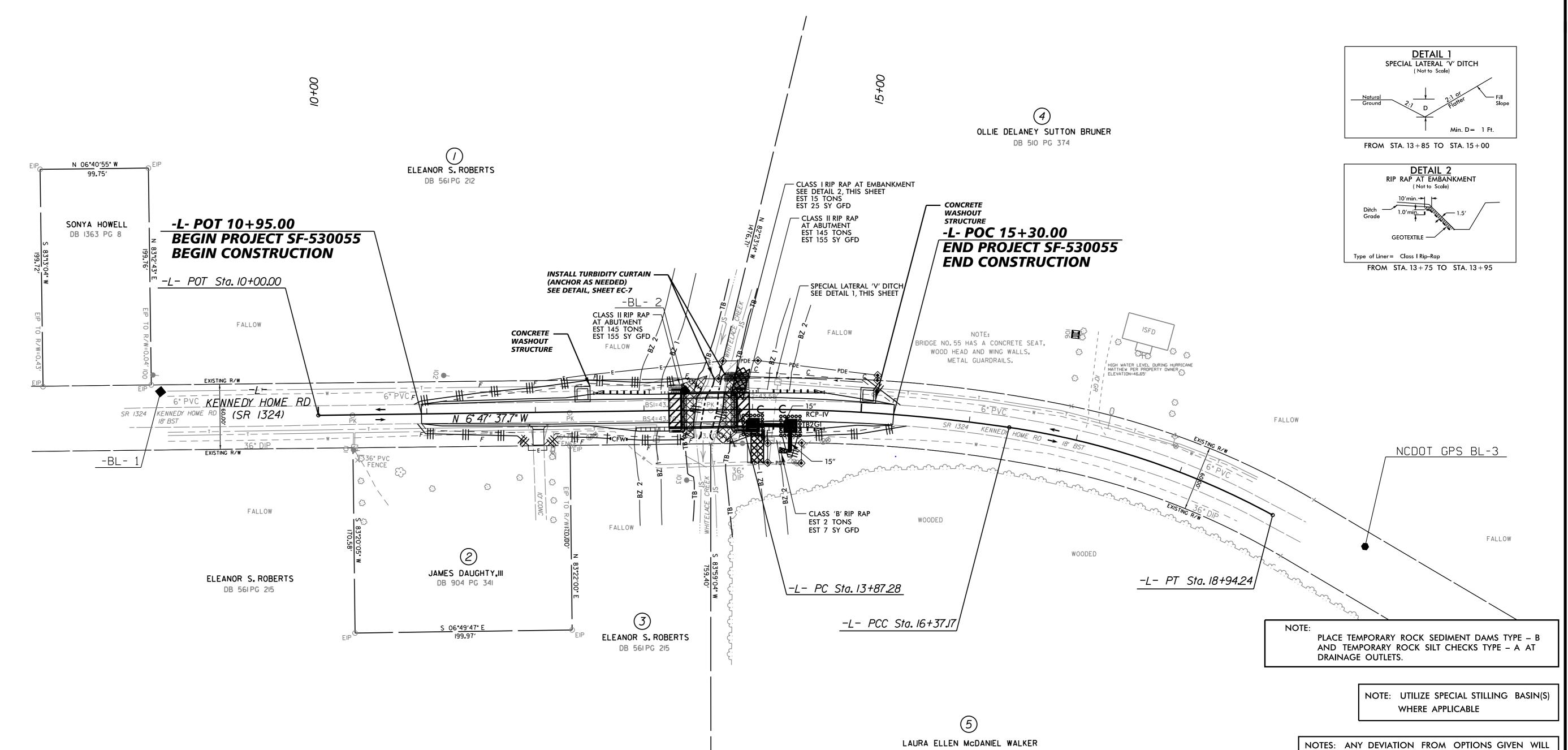
ENVIRONMENTALLY SENSITIVE AREA SEE PROJECT SPECIAL PROVISIONS

EROSION CONTROL PLAN

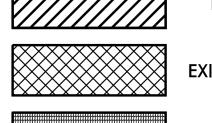
PROJECT REFERENCE NO. SHEET NO. SF -530055 EC-4 R/W SHEET NO. CONST. 4

421 FAYETTEVILLE STREET, SUITE 600 RALEIGH, NC 27601





ENGINEER. BRIDGE APPROACH SLAB



REQUIRE PRIOR APPROVAL BY ENGINEER.

ADDITIONAL EROSION CONTROL DEVICES MAY NEED TO BE INSTALLED AS DIRECTED BY THE



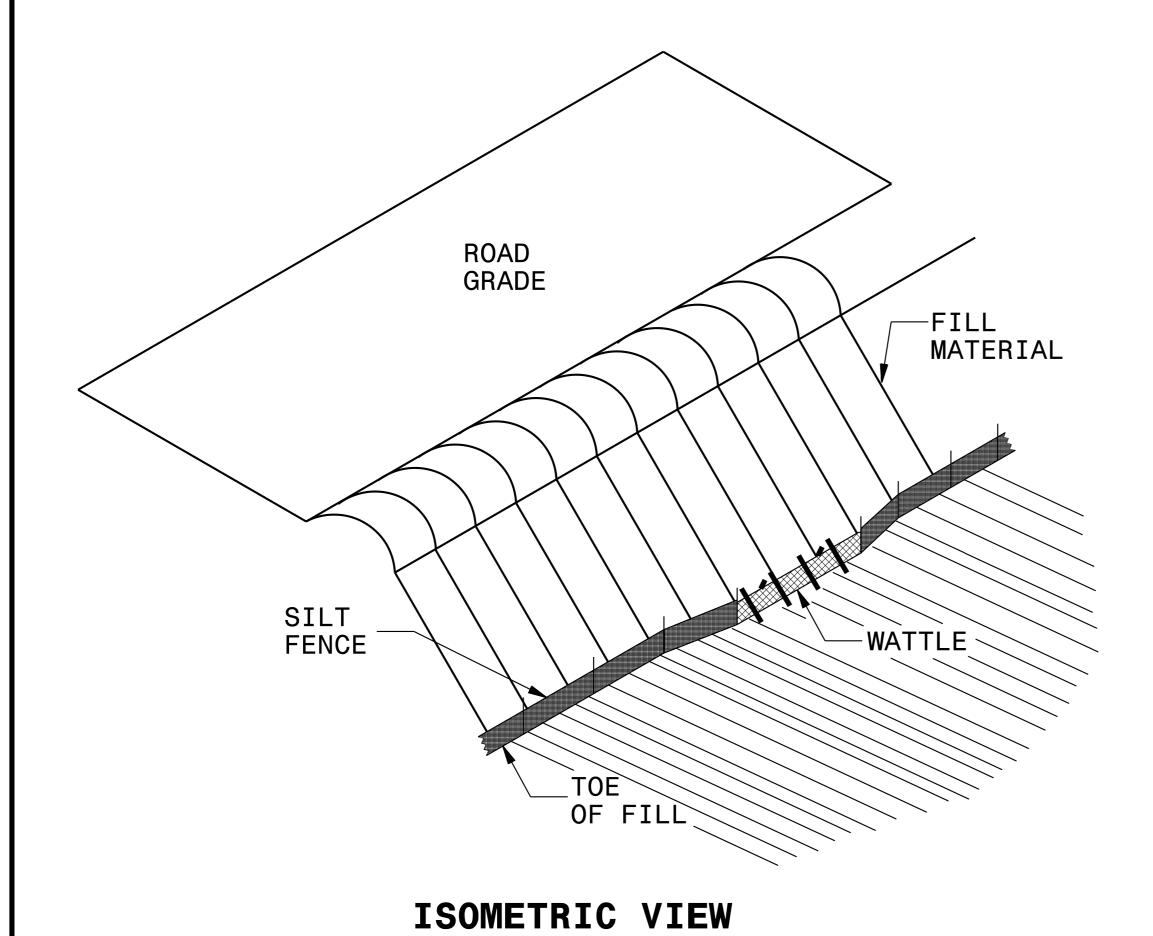
EXIST RDWY. FILL EXCAVATION

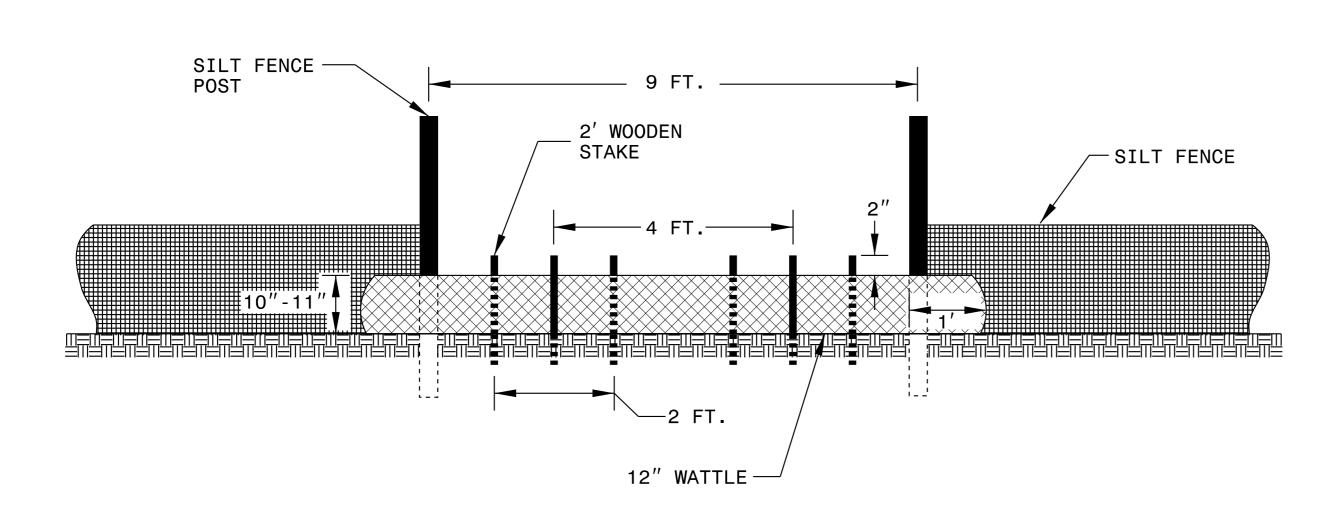
CLASS II RIP RAP

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ROJECT REFERENCE NO. SHEET NO. SF-530055 EC-5 RW SHEET NO.

SILT FENCE COIR FIBER WATTLE BREAK DETAIL





VIEW FROM SLOPE

NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLE ON TOE OF SLOPE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

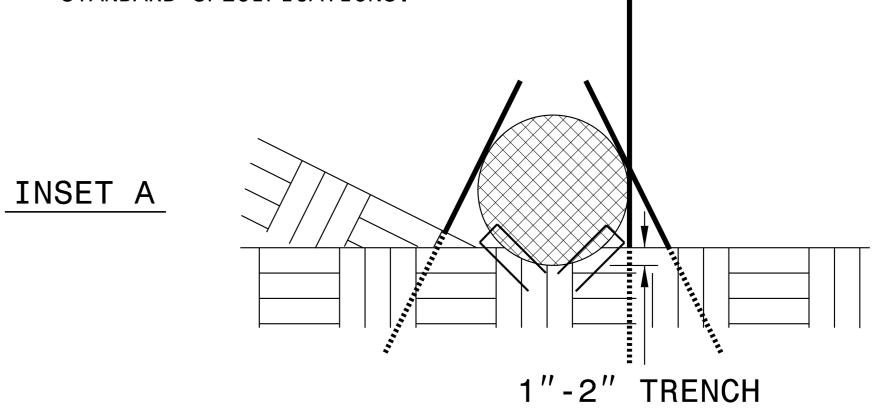
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

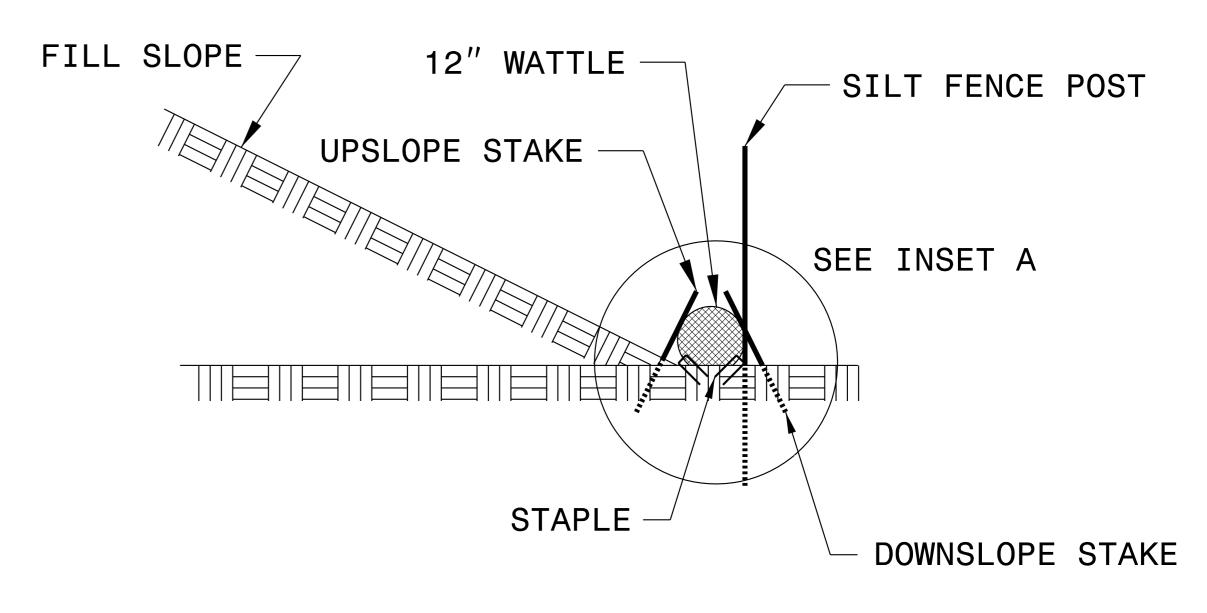
PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

WATTLE INSTALLATION CAN BE ON OUTSIDE OF THE SILT FENCE AS DIRECTED.

INSTALL TEMPORARY SILT FENCE IN ACCORDANCE WITH SECTION 1605 OF THE STANDARD SPECIFICATIONS.





SIDE VIEW

5.2 – Flow Diversion

FLOATING TURBIDITY CURTAIN

USE THIS DEVICE FOR CONSTRUCTION OF THE PROPOSED BULKHEADS WITHIN THE EXISTING WATERWAY.

MATERIAL AND INSTALLATION REFERENCES:

NCDOT - "BMP'S FOR CONSTRUCTION AND MAINTENANCE ACTIVITIES" (AUGUST 2003, PAGES 74 AND 75) SECTION 5.2.5 "TURBIDITY CURTAIN" (http://www.ncdot.org/doh/operations/BMP_manual/download/BMP_Manual.pdf)

NCDOT SPECIAL PROVISION -"FLOATING TURBIDITY CURTAIN" (http://www.ncdot.org/doh/operations/dp_chief_eng/ roadside/soil_water/pdf/FloatingTurbidityCurtain.pdf)

FLOATING TURBIDITY CURTAIN:

This work consists of furnishing a Floating Turbidity Curtain to deter silt suspension and movement of silt particles during construction. The floating turbidity curtain shall be constructed at locations as directed.

The curtain material shall be made of a tightly woven nylon, plastic or other nondeteriorating material meeting the following specifications:

Property Grab tensile strength Mullen burst stength Trapezoid tear strength Apparent opening size Percent open area

*md-370 lbs *cd-250 lbs 480 psi *md-100 lbs *cd-60 lbs 70 US standard sieve 4% permittivity 0.28 sec-1

Pay Unit

Square Yard

*md - machine direction *cd — cross machine direction

In the event that more than one width of fabric is required, a 6" overlap of the material

The curtain material shall be supported by a flotation material having over 29 lbs/ft buoyancy. The floating curtain shall have a 5/16" galvanized chain as ballast and dual 5/16" galvanized wire ropes with a heavy vinyl coating as load lines.

Construction Methods

The Contractor shall maintain the Floating Turbidity Curtain in a satisfactory condition until its removal is requested by the Engineer. The curtain shall extend to the bottom of the jurisdictional resource. Anchor the curtain according to manufacturer recommendations.

Measurement and Payment

Floating Turbidity Curtain will be measured and paid for as the actual number of square yards of curtain furnished as specified and accepted. Such price and payment will be full compensation for the work as described in this section including but not limited to furnishing all materials, tools, equipment, and all incidentals necessary to complete the

Payment will be made under:

Floating Turbidity Curtain

5.2 – Flow Diversion

5.2.5 Turbidity Curtain

Purpose

Used as instream erosion control filtration device to isolate the streambank work from the normal flow of the stream. This device is normally used in open waters for containment in work zones. May also be used across channels with very low flow for shortterm work when anchored properly.



Figure 33. Turbidity Curtain

Conditions Where Practice Applies

- ✓ When performing work on a stream bank in a small localized
- ✓ When the repair or construction activities will not require an extended period of time to complete.

Conditions Where Practice Does Not Apply

✓ Across flowing streams. Turbidity curtains are not designed as prefabricated dams.

Construction

The curtain should be made of a tightly woven nylon, plastic or other non-deteriorating material. The material shall meet the following specifications:

Property
Grab tensile strength <u>Value</u> *md-370 lbs. (1.65 Kn) *cd-250 lbs. (1.11 Kn) Mullen burst strength 480 psi (3307 kpa) Trapezoid tear strength *md-100 lbs. (0.45 Kn) *cd-60 lbs. (0.27 Kn)

Apparent opening size 70 us standard sieve (0.210 mm) Percent open area 4% permittivity 0.28 sec-1

*md – machine direction *cd – cross machine direction

 A flotation material with over 29 lbs./ft. (43 kg/m) buoyancy shall support the curtain material. A 5/16 inch (7.8 mm) galvanized chain shall act as ballast for the floating curtain. Dual 5/16 inch (7.8 mm) galvanized wire ropes with a heavy vinyl coating shall be used as the load lines.

Maintenance

- Inspect the curtain, flotation and ballast to ensure the work area is securely partitioned from the stream flow.
- Remove accumulated sediment and debris.

Typical Problems

- Does not permanently remove sediment.
- Improper anchoring of curtain on channel bottom. • Tidal flows requiring frequent repositioning

North Carolina Department of Transportation

BMPs for Construction and Maintenance Activities

North Carolina Department of Transportation August 2003

BMPs for Construction and Maintenance Activities

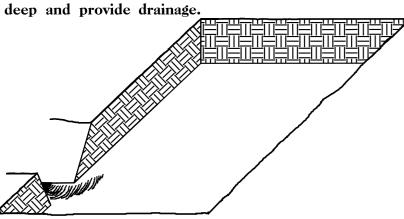
SF-530055

PLANTING DETAILS

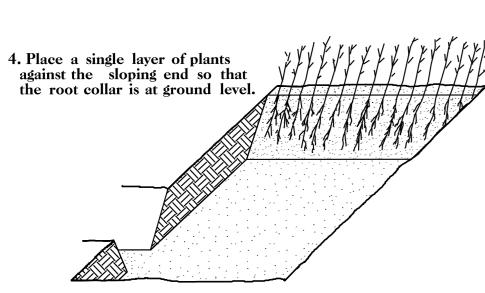
SEEDLING / LINER BAREROOT PLANTING DETAIL

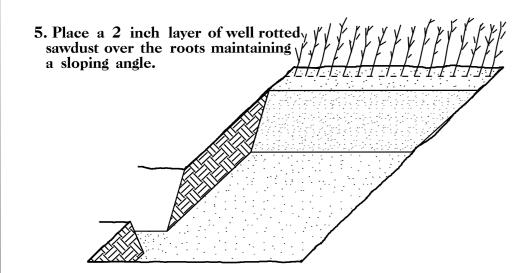
HEALING IN

- 1. Locate a healing-in site in a shady, well protected area.
- 2. Excavate a flat bottom trench 12 inches deep and provide drainage.



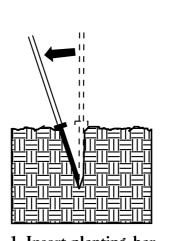
3. Backfill the trench with 2 inches well rotted sawdust. Place a 2 inch layer of well rotted sawdust at a sloping angle at one end of the trench.



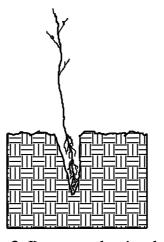


6. Repeat layers of plants and sawdust as necessary and water thoroughly.

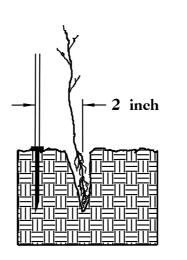
DIBBLE PLANTING METHOD USING THE KBC PLANTING BAR



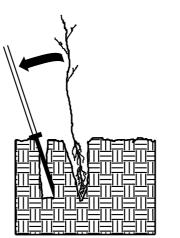
1. Insert planting bar as shown and pull handle



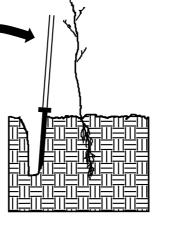
2. Remove planting bar and place seedling at correct depth.



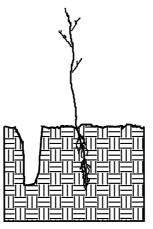
3. Insert planting bar 2 inches toward planter



4. Pull handle of bar toward planter, firming soil at bottom.



5. Push handle forward firming soil at top.



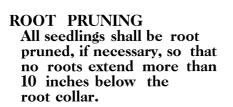
6. Leave compaction hole open. Water thoroughly.

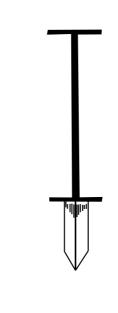
PLANTING NOTES:

PLANTING BAG During planting, seedlings shall be kept in a moist canvas bag or similar container to prevent the root systems from drying.



KBC PLANTING BAR
Planting bar shall have a
blade with a triangular cross section, and shall be 12 inches long, 4 inches wide and 1 inch thick at center.





REFORESTATION

 \square TREE REFORESTATION SHALL BE PLANTED 6 FT. TO 10 FT. ON CENTER, RANDOM SPACING, AVERAGING 8 FT. ON CENTER, APPROXIMATELY 680 PLANTS PER ACRE.

REFORESTATION

MIXTURE, TYPE, SIZE, AND FURNISH SHALL CONFORM TO THE FOLLOWING:

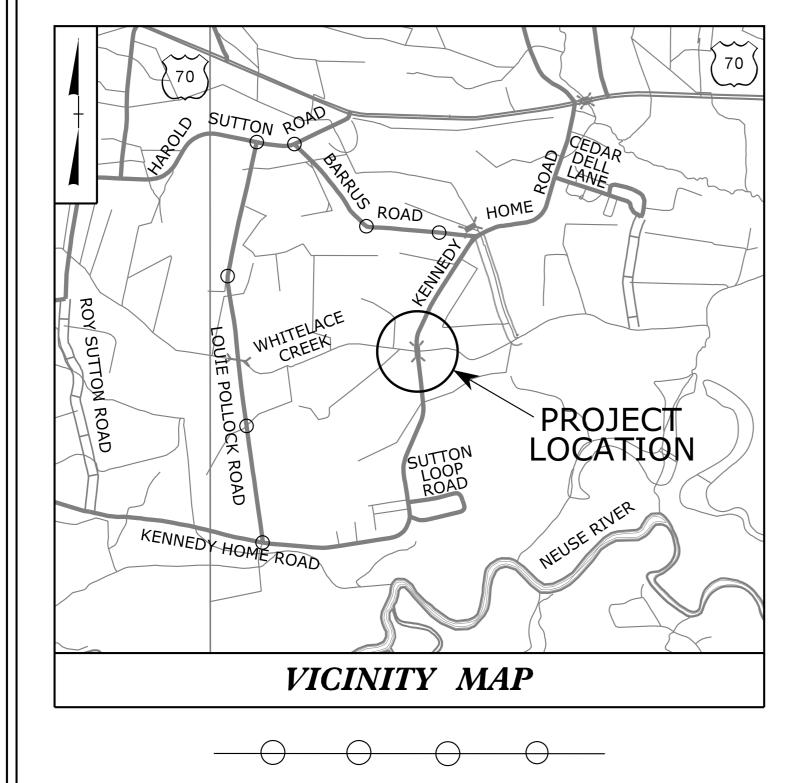
25% LIRIODENDRON TULIPIFERA	TULIP POPLAR	12 in - 18 in BR
25% PLATANUS OCCIDENTALIS	AMERICAN SYCAMORE	12 in - 18 in BR
25% FRAXINUS PENNSYLVANICA	GREEN ASH	12 in - 18 in BR
25% BETULA NIGRA	RIVER BIRCH	12 in - 18 in BR

REFORESTATION DETAIL SHEET

N.C.D.O.T. - ROADSIDE ENVIRONMENTAL UNIT

PROJECT: SF-530055

See Sheet 1A For Index of Sheets See Sheet 1B For Conventional Symbols



OFF SITE DETOUR ROUTE

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

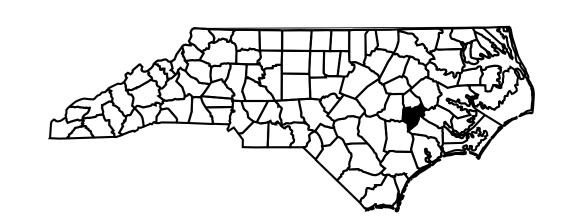
LENOIR COUNTY

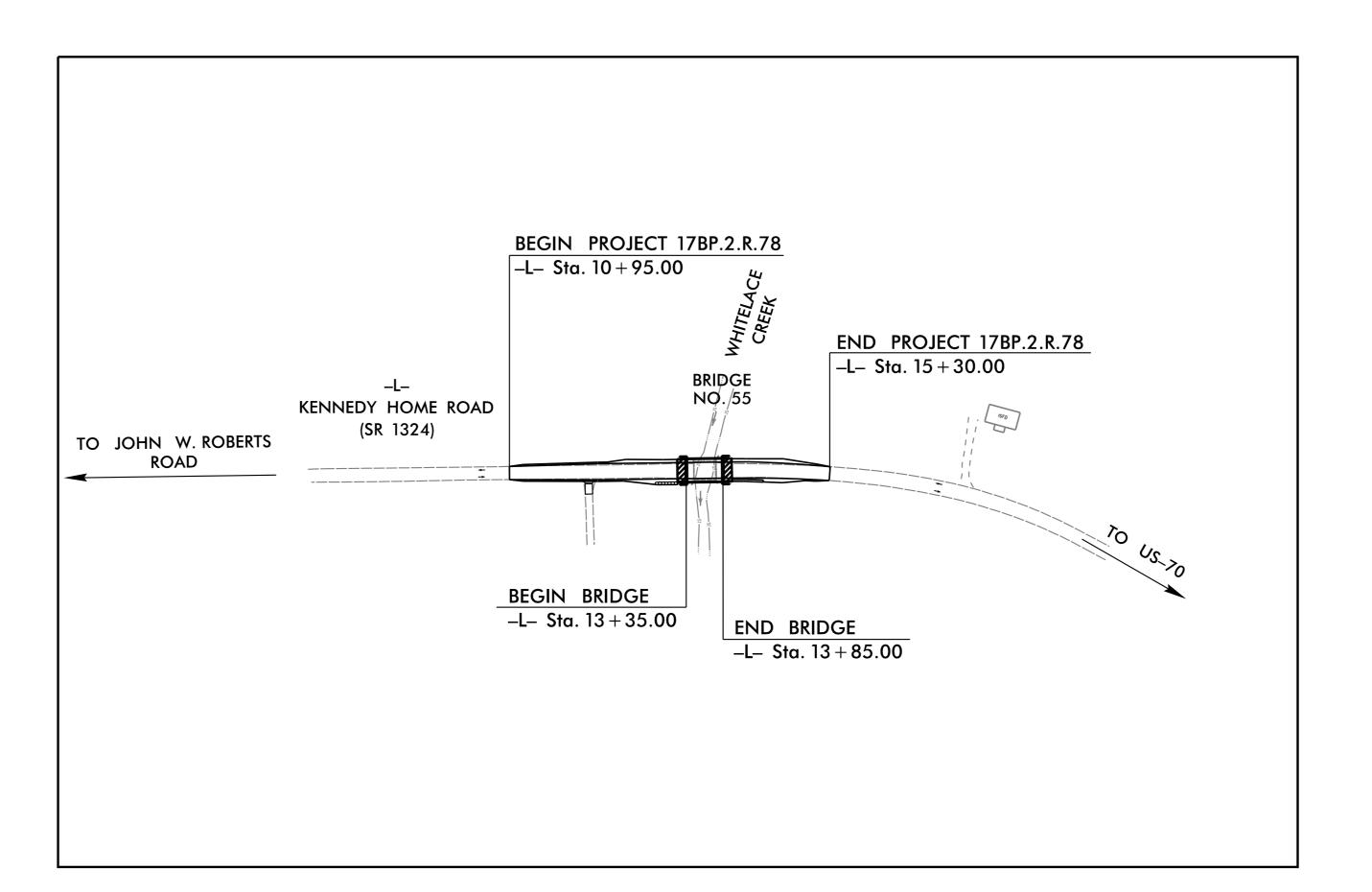
LOCATION: BRIDGE NO. 55 OVER WHITELACE CREEK ON SR 1324 (KENNEDY HOME ROAD)

TYPE OF WORK: GRADING, DRAINAGE, PAVING, AND STRUCTURE

STATE	STATE F	PROJECT REFERENCE NO.	SHEET	SHEETS
N.C.	SF	-530055	U0-1	
STAT	E PROJ. NO.	F. A. PROJ. NO.	DESCRIPT	ION
17BF	P.2.R.78			

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED







CONTRACT

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INDEX OF SHEETS

SHEET NO.

UO-1 UO-2 DESRIPTION
TITLE SHEET

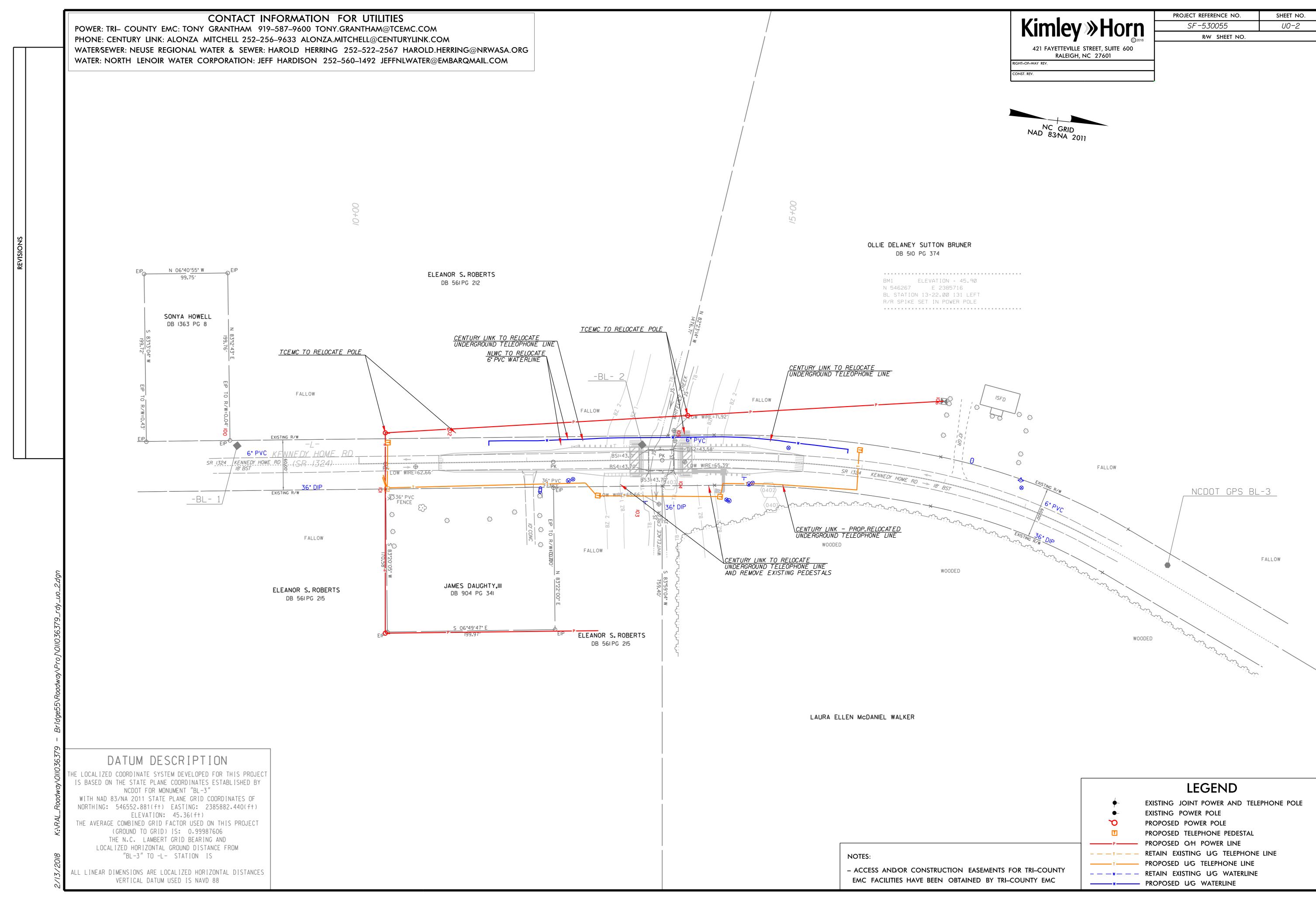
UTILITY BY OTHER PLAN SHEETS

UTILITY OWNERS ON PROJECT

- (1) TRI-COUNTY EMC (POWER)
- (2) NORTH LENOIR WATER CORPORATION (WATER)
- (3) NEUSE REGIONAL WATER AND SEWER AUTHORITY (WATER)
- (4) CENTURY LINK (PHONE)

UTILITY PLANS BY:

NCDOT PROJECT ENGINEER:
HEATHER C. LANE, P.E.
PREPARED FOR:
NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION BRIDGE PROGRAM
DIVISION 2

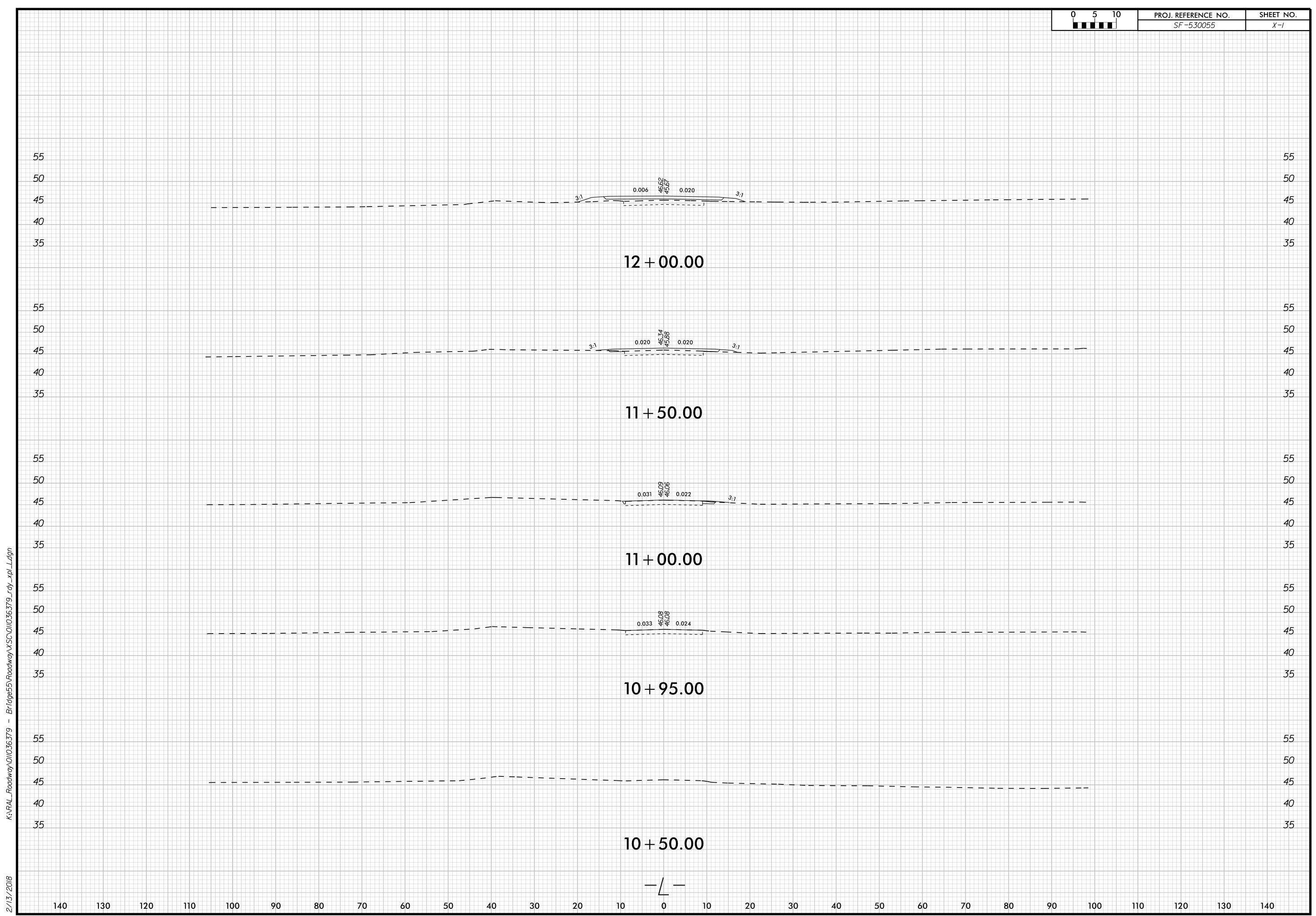


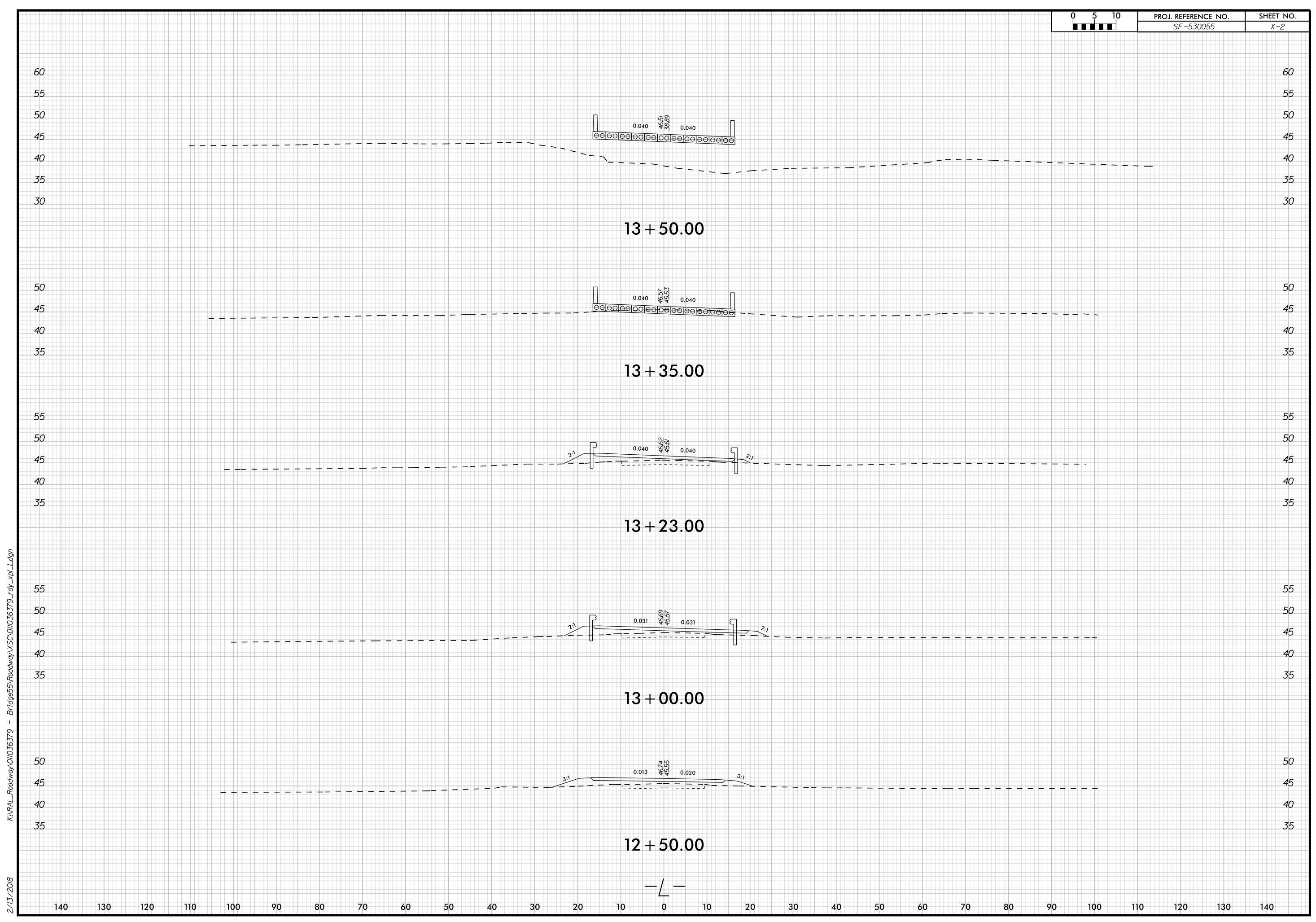
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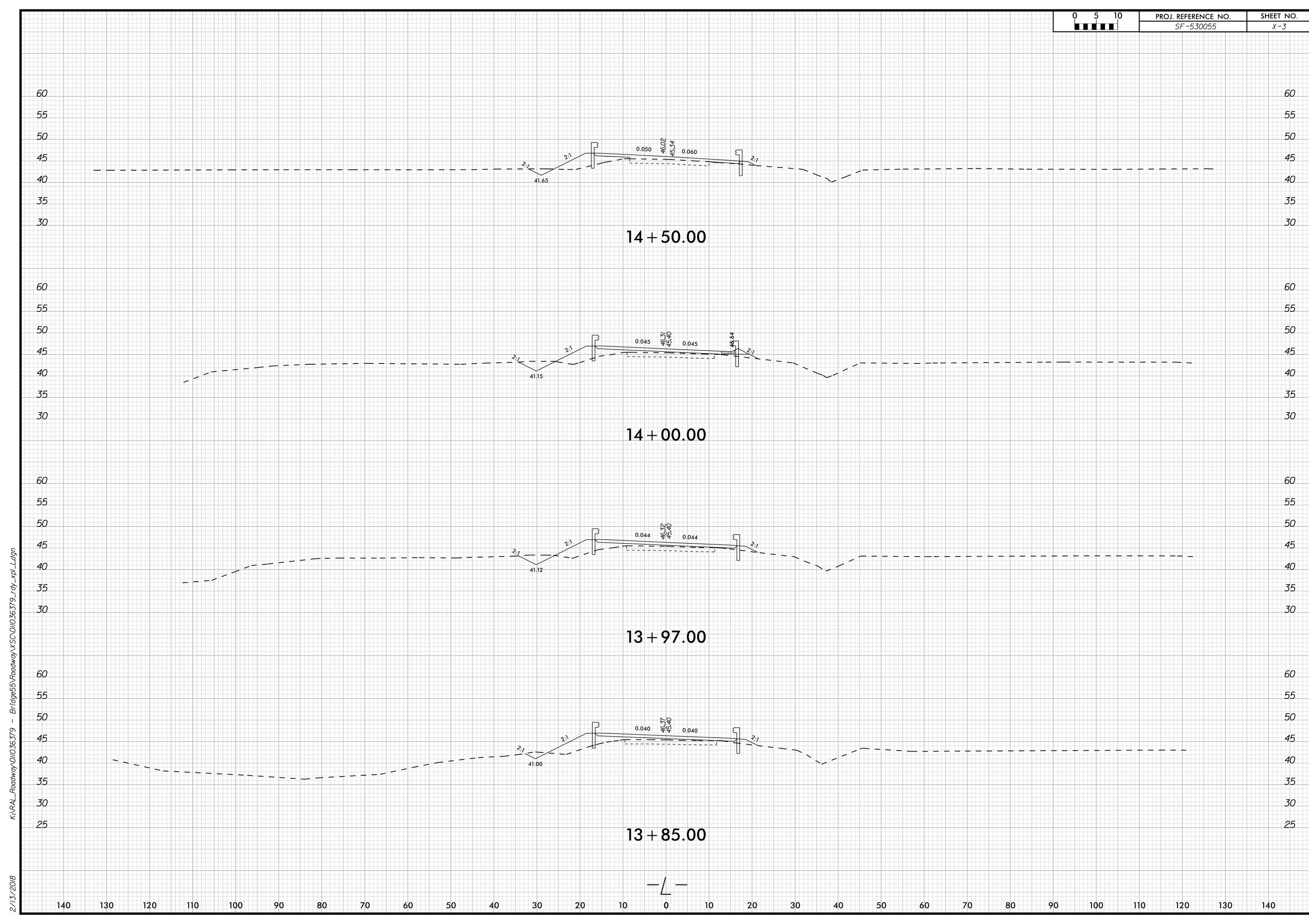
PROJ. REFERENCE NO.SHEET NO.SF-530055X-IA

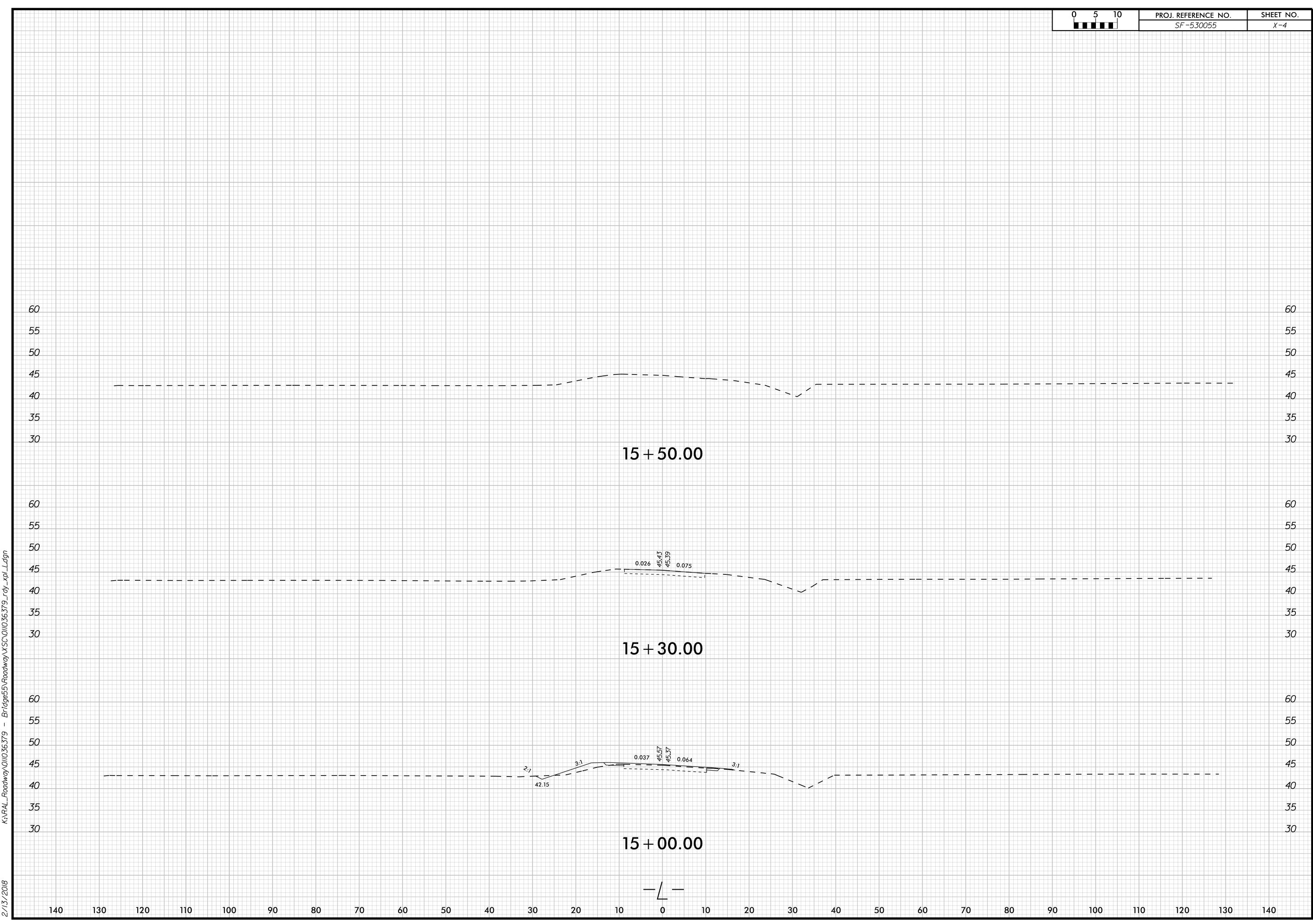
SF-530055 - BRIDGE 55 REPLACEMENT CROSS SECTION INDEX

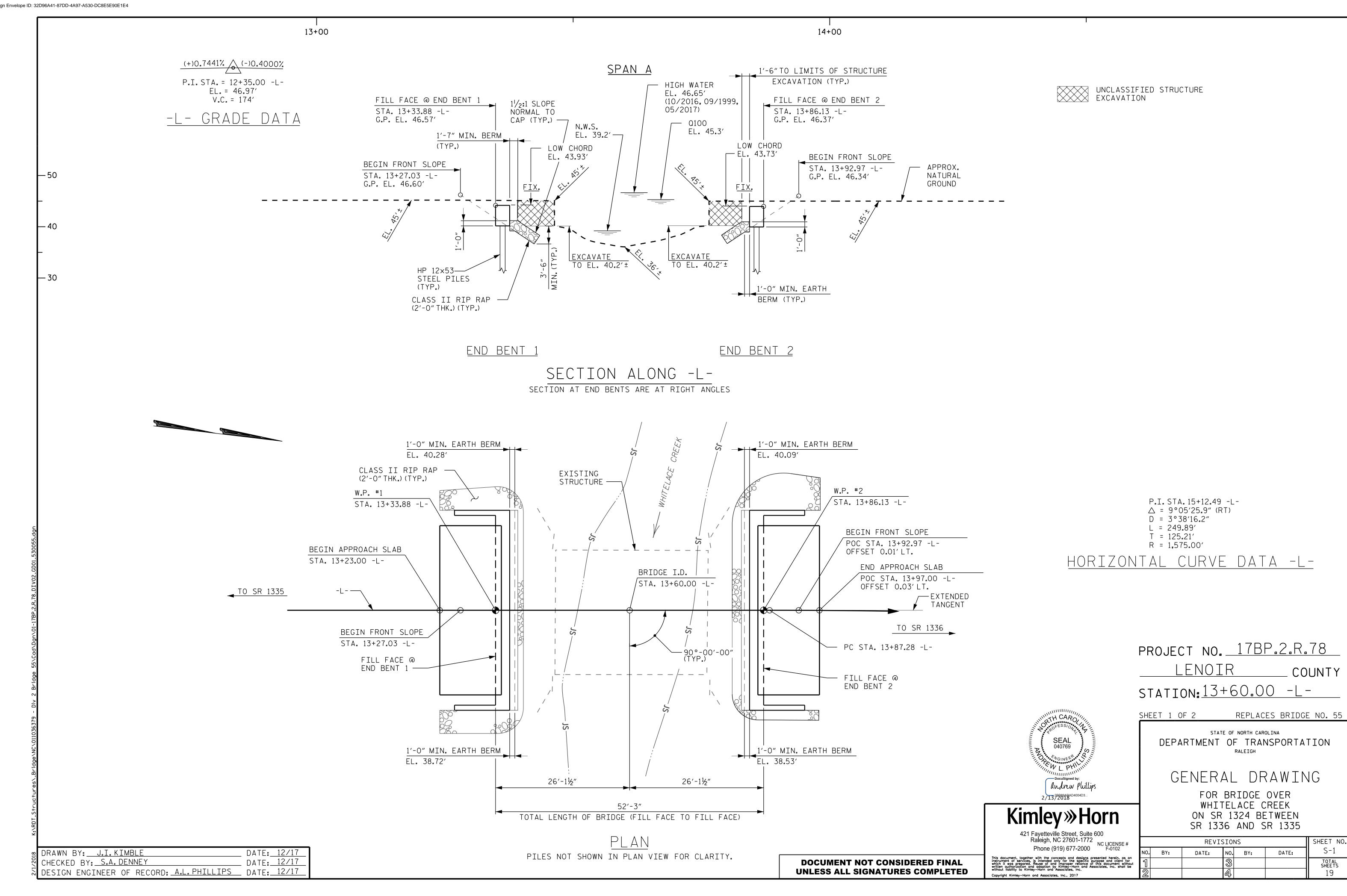
-L- SR 1324 (KENNEDY HOME RD) X-1 THRU X-4

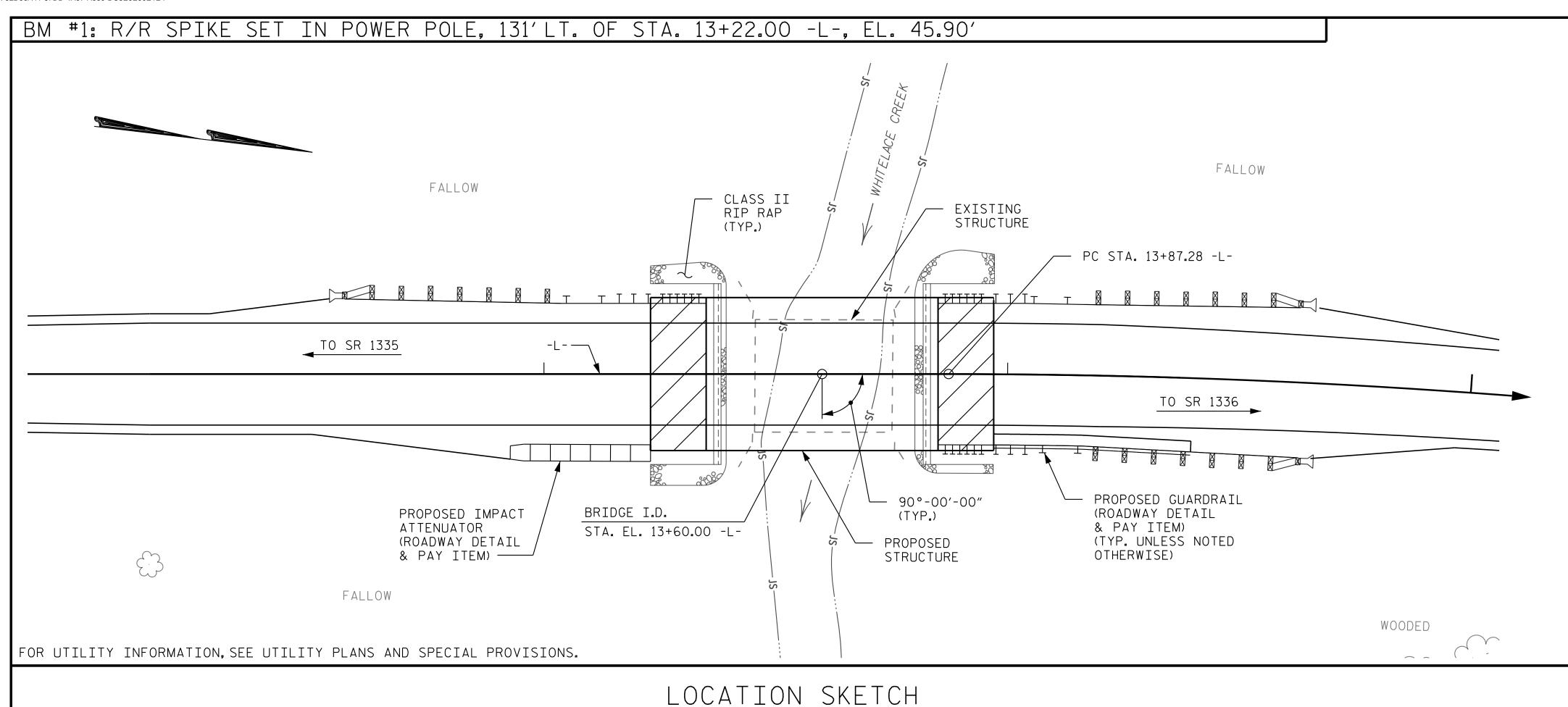












NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES".

THE EXISTING STRUCTURE CONSISTING OF 1 SPAN @ 31'-0" WITH A CLEAR ROADWAY OF APPROX. 24.5' ON PRESTRESSED CONCRETE CHANNELS ON PRECAST PRESTRESSED CONCRETE CAPS ON TIMBER PILES AND LOCATED AT THE PROPOSED STRUCTURE, SHALL BE REMOVED. THE EXISTING BRIDGE IS CURRENTLY NOT POSTED.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COSTS INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET S-1 SHALL BE EXCAVATED FOR A DISTANCE OF 30 FT.EACH SIDE OF & ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

ASPHALT WEARING COURSE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES. SEE SPECIAL PROVISIONS.

						— TO	TAL BI	LL OF MA	TEF	RIAL								
	REMOVAL OF EXISTING STRUCTURE	ASBESTOS ASSESSMENT	PDA TESTING	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	PILE DRIVING EQUIPMENT SETUP FOR HP 12 X 53 STEEL PILES	HP STEE	12 X 53 L PILES	<u>' </u>	TWO BAR METAL RAIL	1'-2" X 2'-8¾" CONCRETE PARAPET	RIP RAP CLASS II (2'-0"THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIO BEARINGS	PRES CON	"X 1'-9" STRESSED NCRETE ID SLABS
	LUMP SUM	LUMP SUM	EACH	LUMP SUM	CU.YDS.	LUMP SUM	LBS.	EACH	NO.	LIN"FT"	EACH	LIN.FT.	LIN.FT.	TON	SQ.YD.	LUMP SUM	NO.	LIN.FT.
SUPERSTRUCTURE						LUMP SUM						97.00	112.00			LUMP SUM	11	550
END BENT 1				LUMP SUM	21.6		2,638	7	7	350	4			52	58			
END BENT 2				LUMP SUM	21.6		2,638	7	7	350	4			64	71			
TOTAL	LUMP SUM	LUMP SUM	1	LUMP SUM	43.2	LUMP SUM	5,276	14	14	700	8	97.00	112.00	116	129	LUMP SUM	11	550

FOUNDATION NOTES

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO. 1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 71 TONS PER PILE.

PILES AT END BENT NO. 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 71 TONS PER PILE.

DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 120 TONS PER PILE.

DRIVE PILES AT END BENT NO. 2 TO A REQUIRED DRIVING RESISTANCE OF 120 TONS PER PILE.

TESTING PILES WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING MAY BE REQUIRED. THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

DATE: 12/17 DRAWN BY: J.I.KIMBLE DATE: 12/17 CHECKED BY: S.A. DENNEY DESIGN ENGINEER OF RECORD: <u>A.L.PHILLIPS</u> DATE: <u>12/17</u>

HYDRAULIC DATA

DESIGN DISCHARGE ----- 740 C.F.S. FREQUENCY OF DESIGN FLOOD ----- 25 YRS. DESIGN HIGH WATER ELEVATION---- 44.5 FT. DRAINAGE AREA ------ 4.92 SQ. MI. BASIC DISCHARGE (Q100)----- 1120 C.F.S. BASIC HIGH WATER ELEVATION----- 45.3 FT.

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE ----- 1300 C.F.S. FREQUENCY OF OVERTOPPING FLOOD --- >100 YRS. OVERTOPPING FLOOD ELEVATION ----- 45.7 FT.



421 Fayetteville Street, Suite 600
Raleigh, NC 27601-1772
NC LICENSE #
F-0102

PROJECT NO. 17BP.2.R.78 LENOIR COUNTY STATION: 13+60.00 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING

FOR BRIDGE OVER WHITELACE CREEK ON SR 1324 BETWEEN SR 1336 AND SR 1335

SHEET NO REVISIONS S-2 DATE: NO. BY: DATE: BY: TOTAL SHEETS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

										STRE	ENGTH	I LIN	MIT ST	TATE				SE	RVICE	III	LIMI	T STA	TE	
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	1.394		1.75	0.276	1.57	50′	EL	24.5	0.531	1.39	50′	EL	2.45	0.80	0.276	1.44	50′	EL	24.5	
DESIGN		HL-93(0pr)	N/A		1.807		1.35	0.276	2.03	50′	EL	24.5	0.531	1.81	50′	EL	2.45	N/A						
LOAD RATING		HS-20(Inv)	36.000	2	1.667	60.007	1.75	0.276	1.95	50′	EL	24.5	0.531	1.67	50′	EL	2.45	0.80	0.276	1.79	50′	EL	24.5	
KATING		HS-20(0pr)	36.000		2.161	77.787	1.35	0.276	2.52	50′	EL	24.5	0.531	2.16	50′	EL	2.45	N/A						
		SNSH	13.500		3.635	49.079	1.4	0.276	4.95	50′	EL	24.5	0.531	4.7	50′	EL	2.45	0.80	0.276	3.64	50′	EL	24.5	
		SNGARBS2	20.000		2.871	57.42	1.4	0.276	3.91	50′	EL	24.5	0.531	3.42	50′	EL	2.45	0.80	0.276	2.87	50′	EL	24.5	
		SNAGRIS2	22.000		2.778	61.109	1.4	0.276	3.78	50′	EL	19.6	0.531	3.21	50′	EL	2.45	0.80	0.276	2.78	50′	EL	24.5	
		SNCOTTS3	27.250		1.814	49.418	1.4	0.276	2.47	50′	EL	24.5	0.531	2.36	50′	EL	2.45	0.80	0.276	1.81	50′	EL	24.5	
	\ \cdot \[\]	SNAGGRS4	34.925		1.577	55.063	1.4	0.276	2.15	50′	EL	24.5	0.531	2.01	50′	EL	2.45	0.80	0.276	1.58	50′	EL	24.5	
		SNS5A	35.550		1.537	54.657	1.4	0.276	2.09	50′	EL	24.5	0.531	2.07	50′	EL	2.45	0.80	0.276	1.54	50′	EL	24.5	
		SNS6A	39.950		1.438	57.43	1.4	0.276	1.96	50′	EL	24.5	0.531	1.91	50′	EL	2.45	0.80	0.276	1.44	50′	EL	24.5	
LEGAL		SNS7B	42.000		1.370	57.54	1.4	0.276	1.87	50′	EL	24.5	0.531	1.91	50′	EL	2.45	0.80	0.276	1.37	50′	EL	24.5	
LOAD		TNAGRIT3	33.000		1.761	58.118	1.4	0.276	2.4	50′	EL	24.5	0.531	2.25	50′	EL	2.45	0.80	0.276	1.76	50′	EL	24.5	
RATING		TNT4A	33.075		1.777	58.759	1.4	0.276	2.42	50′	EL	24.5	0.531	2.17	50′	EL	2.45	0.80	0.276	1.78	50′	EL	24.5	
		TNT6A	41.600		1.480	61.558	1.4	0.276	2.01	50′	EL	24.5	0.531	2.08	50′	EL	2.45	0.80	0.276	1.48	50′	EL	24.5	
	ST	TNT7A	42.000		1.502	63.087	1.4	0.276	2.05	50′	EL	24.5	0.531	1.94	50′	EL	2.45	0.80	0.276	1.50	50′	EL	24.5	
		TNT7B	42.000		1.566	65.773	1.4	0.276	2.13	50′	EL	24.5	0.531	1.84	50′	EL	2.45	0.80	0.276	1.57	50′	EL	24.5	
		TNAGRIT4	43.000		1.486	63.902	1.4	0.276	2.02	50′	EL	24.5	0.531	1.77	50′	EL	2.45	0.80	0.276	1.49	50′	EL	24.5	
		TNAGT5A	45.000		1.388	62.47	1.4	0.276	1.89	50′	EL	24.5	0.531	1.8	50′	EL	2.45	0.80	0.276	1.39	50′	EL	24.5	
		TNAGT5B	45.000	3	1.360	61.206	1.4	0.276	1.85	50′	EL	24.5	0.531	1.68	50′	EL	2.45	0.80	0.276	1.36	50′	EL	24.5	

LOAD FACTORS:

DESIGN LOAD RATING FACTORS SERVICE III 1.00 1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

2

3

4.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

 $\left(\begin{array}{c} \overline{3} \end{array}\right)$ Legal load rating **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. 17BP.2.R.78

LENOIR COUNTY

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

STANDARD

STATION: 13+60.00 -L-



Indrew Phillips
71388209284004D3...

Kimley » Horn

421 Fayetteville Street, Suite 600
Raleigh, NC 27601-1772
Phone (919) 677-2000

NC LICENSE #
F-0102

LRFR SUMMARY FOR 50' CORED SLAB UNIT 90° SKEW (NON-INTERSTATE TRAFFIC)

REVISIONS

BY: DATE: NO. BY: DATE: S-3

TOTAL SHEETS
19

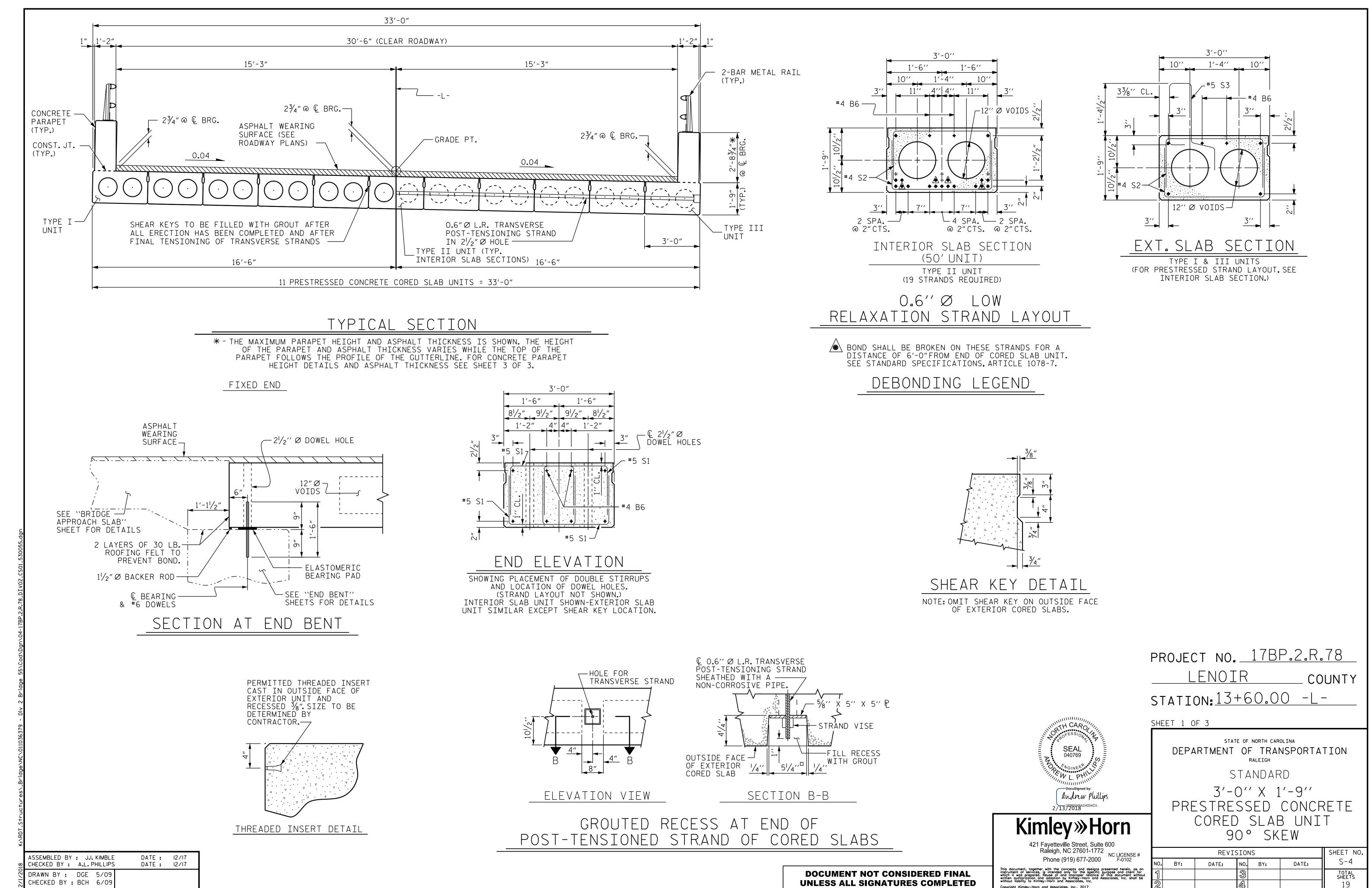
<u>1</u> <u>2</u> <u>3</u>

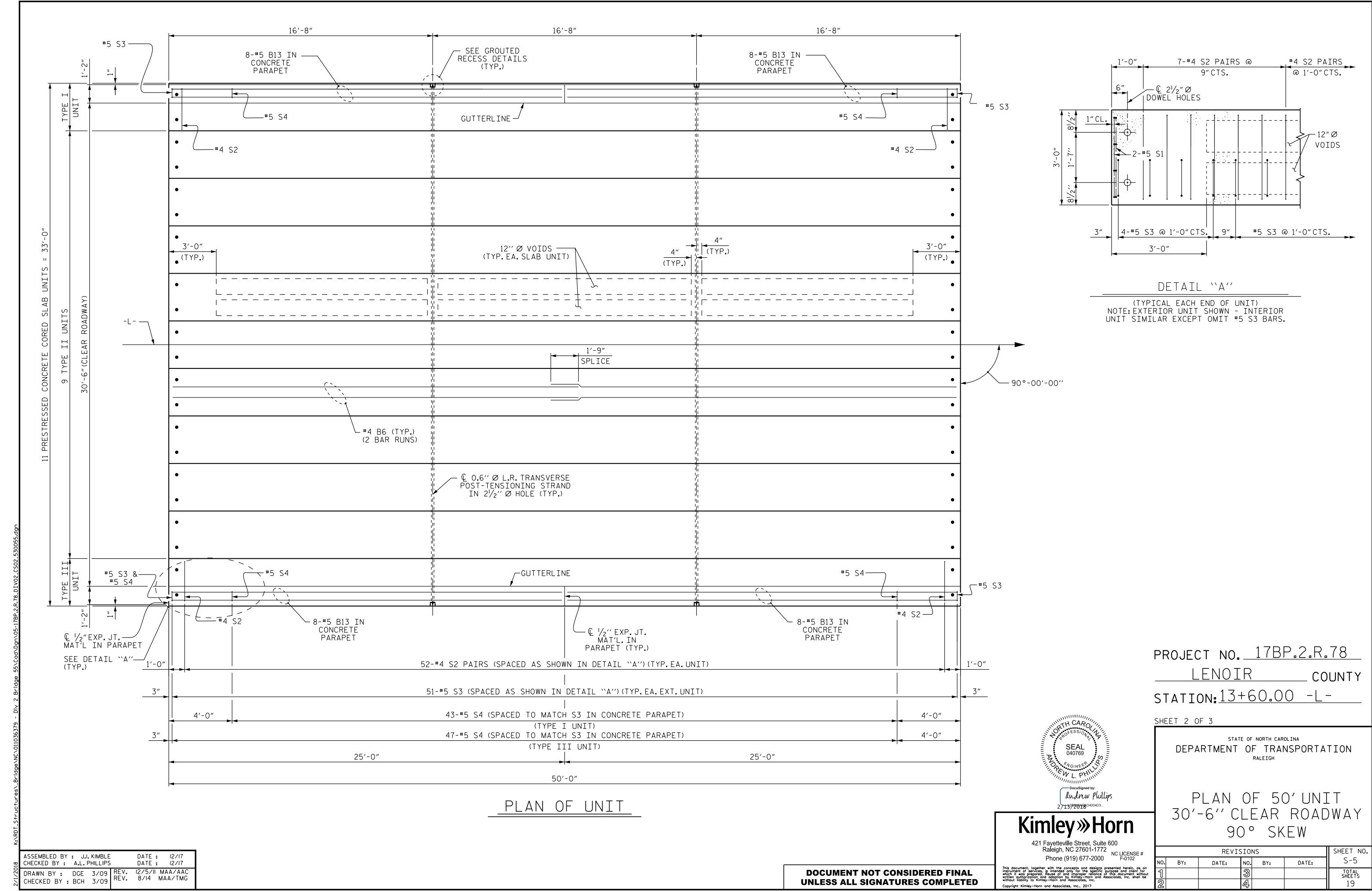
LRFR SUMMARY
FOR SPAN 'A'

ASSEMBLED BY: J.I. KIMBLE DATE: 12/17
CHECKED BY: A.L. PHILLIPS DATE: 12/17

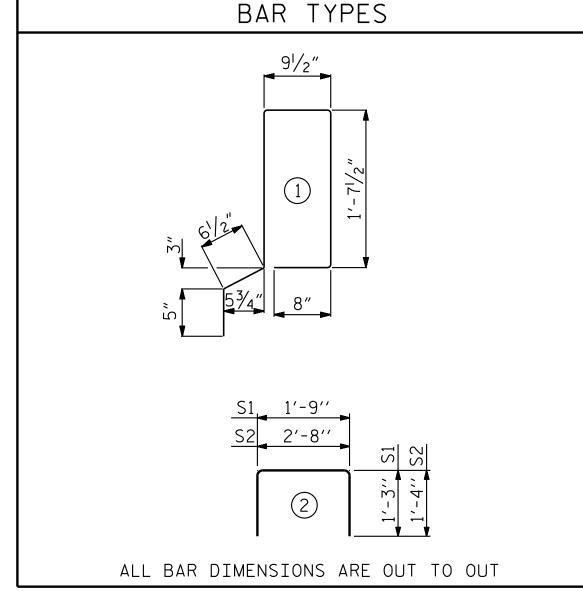
DRAWN BY: CVC 6/10
CHECKED BY: DNS 6/10

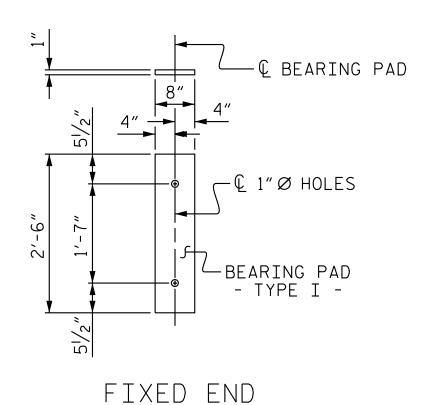
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED





	BILL OF MATERIAL FOR ONE 50'CORED SLAB UNIT											
	TYPE I OR III TYPE II											
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT					
В6	4	#4	STR	25′-9″	69	25′-9″	69					
S1	8	#5	2	4'-3"	35	4'-3"	35					
S2	104	#4	2	5′-4″	371	5′-4″	371					
* S3	51	# 5	1	5′-8″	301							
REINFO	ORCING :	STEEL	LBS	5.	475		475					
	Y COATE											
	IFORCINO				301							
6500 F	P.S.I. CO	NCRETE	CU. YDS) <u>.</u>	7.1		7.1					
0.6"Ø	L.R. STR	ANDS	No).	19		19					





ELASTOMERIC BEARING DETAILS

(TYPE I - 22 REQ'D)

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS.

GUTTERLINE ASPH	HALT THICKNESS & RAI	L HEIGHT
	ASPHALT OVERLAY THICKNESS	PARAPET HEIGHT
	@ MID-SPAN	@ MID-SPAN
50'UNITS	15/8"	2′-75⁄8″

CORED	SLABS	REQI	UIRED
	NUMBER	LENGTH	TOTAL LENGTH
50'UNIT			
TYPE I UNIT	1	50′-0″	50'-0"
TYPE III UNIT	1	50′-0″	50′-0″
TYPE II UNIT	9	50'-0"	450'-0"
TOTAL	11		550′-0″

CONCRETE RELEA	SE STRENGTH
50'UNITS	4900 PSI

GRADE 270 STRANDS				
	0.6″Ø L.R.			
AREA (SQUARE INCHES)	0.217			
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600			
APPLIED PRESTRESS (LBS.PER STRAND)	43,950			

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 1'-9"
50'CORED SLAB UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	1 ¹ /2″ ♦
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD***	3⁄8″ ♦
FINAL CAMBER	11/8"
AND THE THE THE THE THE THE THE	

** INCLUDES FUTURE WEARING SURFACE

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

PRESTRESSED CONCRETE CORED SLABS.

THE $2^{1}/2^{n}$ Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN PARAPETS AND END POSTS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

SHEET 3 OF 3

PROJECT NO. 17BP.2.R.78 LENOIR COUNTY STATION: 13+60.00 -L-

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

andrew Phillips

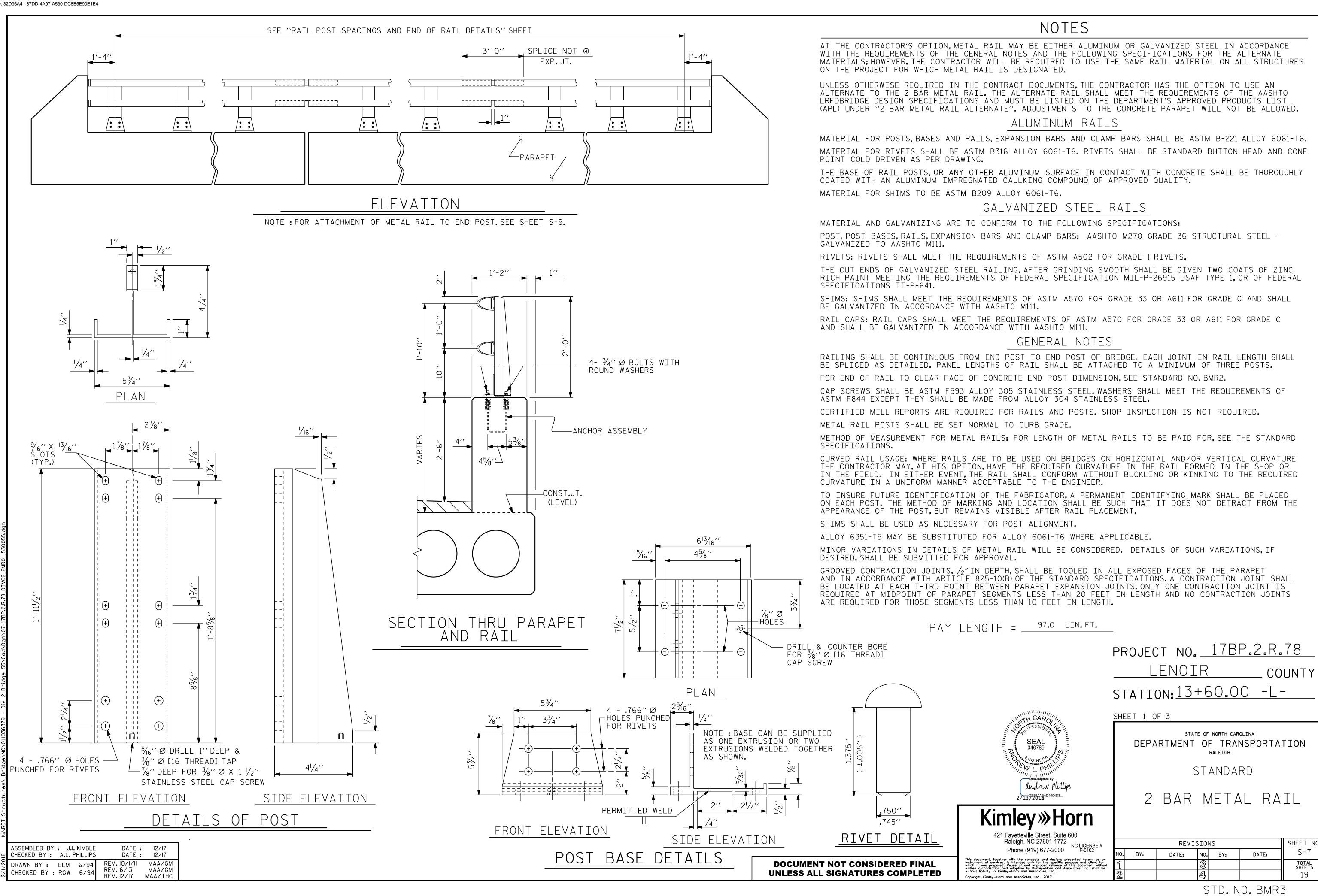
421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 NC LICENSE # F-0102

3'-0'' X 1'-9'' PRESTRESSÉD CONCRETE CORED SLAB UNIT 90° SKEW

REVISIONS SHEET NO S-6 DATE: NO. BY: DATE: BY: TOTAL SHEETS

ASSEMBLED BY : J.I. KIMBLE CHECKED BY : A.L. PHILLIPS DATE : 12/17 DRAWN BY: DGE 5/09
CHECKED BY: BCH 6/09
REV. 8/14
MAA/TMG

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



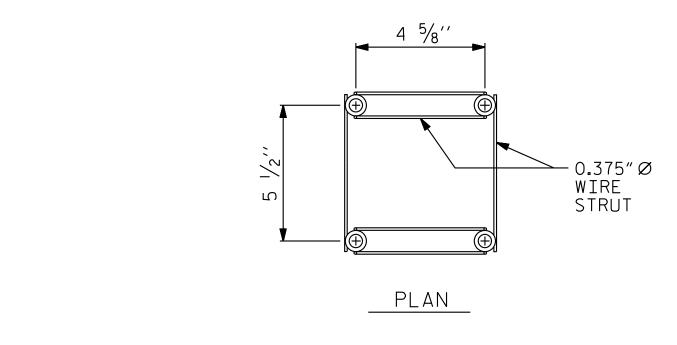
ASSEMBLED BY : J.I. KIMBLE CHECKED BY : A.L. PHILLIPS

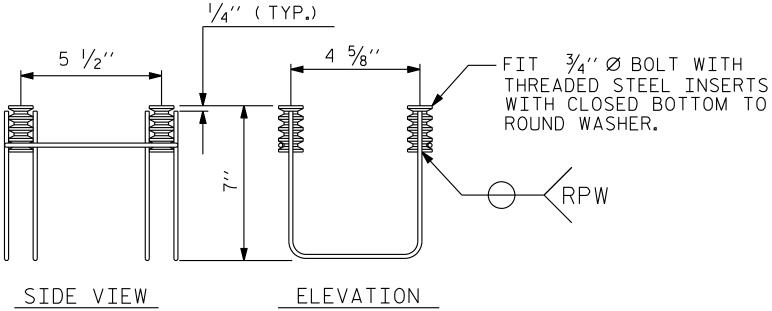
DRAWN BY : EEM 6/94

CHECKED BY : RGW 6/94

DATE : 12/17 DATE : 12/17

REV.5/I/O6R KMM/GM REV.10/I/II MAA/GM REV.12/17 MAA/THC





4-BOLT METAL RAIL ANCHOR ASSEMBLY

(22 ASSEMBLIES REQUIRED)

NOTES

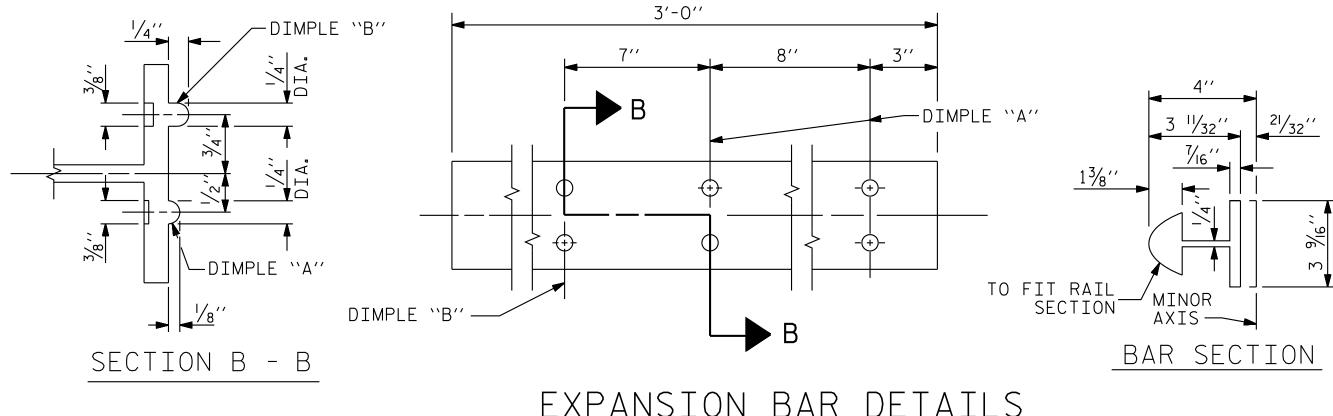
STRUCTURAL CONCRETE ANCHOR ASSEMBLY

THE STRUCTURAL CONCRETE ANCHOR ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

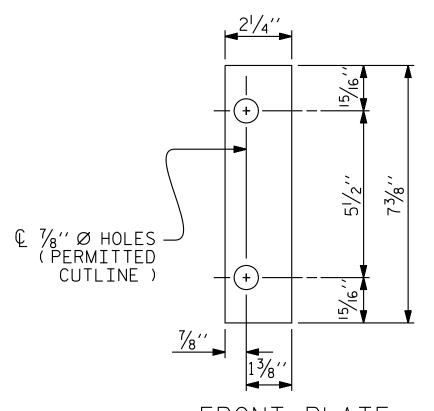
- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 2" FOR 3/4" FERRULES.
- B. 4 $\frac{3}{4}$ " Ø X $2\frac{1}{2}$ " BOLTS WITH WASHERS. BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED.
 AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE $\frac{3}{4}$ " Ø X $2\frac{1}{2}$ " GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.
- C. WIRE STRUT SHOWN IN THE CONCRETE ANCHOR ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A $\frac{7}{16}$ " Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.
- D. THE METAL RAIL ANCHOR ASSEMBLIES TO BE HOT DIPPED GALVANIZED TO CONFORM TO REQUIREMENTS OF AASHTO M111.
- E. THE COST OF THE METAL RAIL ANCHOR ASSEMBLY WITH BOLTS AND WASHERS COMPLETE IN PLACE SHALL BE INCLUDED IN THE PRICE BID FOR LINEAR FEET OF METAL RAIL.
- F. BOLTS TO BE TIGHTENED ONE-HALF TURN WITH A WRENCH FROM A FINGER-TIGHT POSITION.

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF THE METAL RAIL ANCHOR ASSEMBLY. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE $\frac{3}{4}$ " \varnothing BOLT IS 10 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE THE STANDARD SPECIFICATIONS.

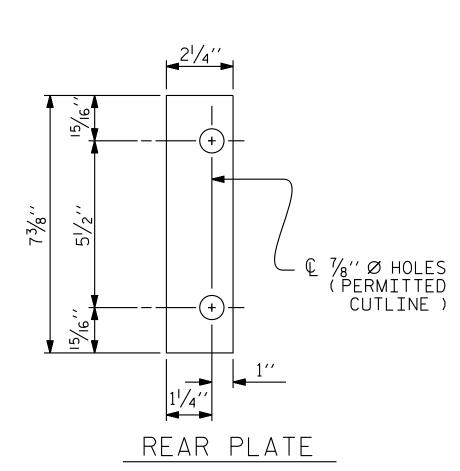
WHEN ADHESIVELY ANCHORED ANCHOR BOLTS ARE USED, BOLTS SHALL MEET THE REQUIREMENTS OF ASTM F593 ALLOY 304 STAINLESS STEEL WITH MINIMUM 75,000 PSI ULTIMATE STRENGTH. NUTS SHALL MEET THE REQUIREMENTS OF ASTM F594 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.



EXPANSION BAR DETAILS

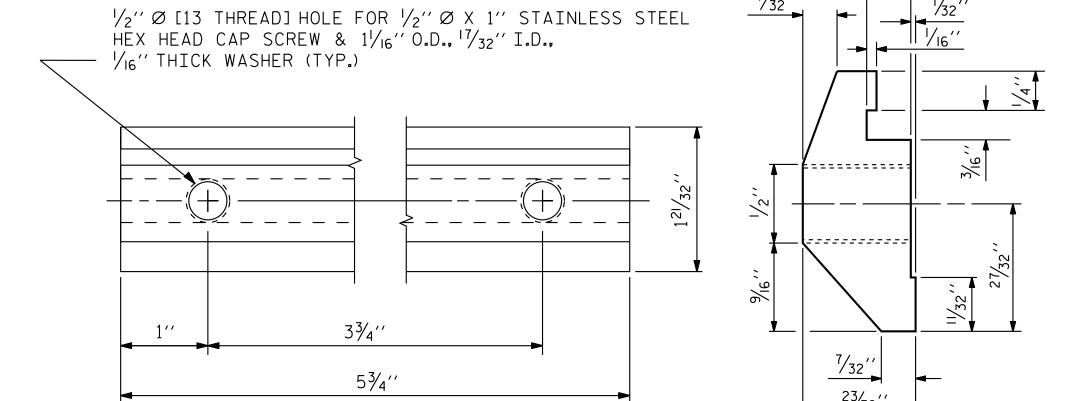


FRONT PLATE

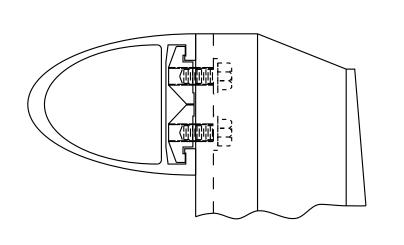


SHIM DETAILS

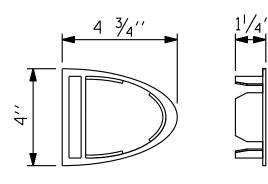
NOTE : SHIMS MAY BE CUT ALONG PERMITTED CUTLINE OR SLOTTED TO EDGE OF PLATE TO FACILITATE PLACEMENT.



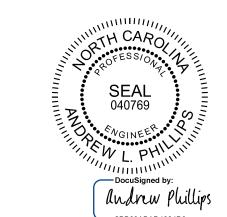
AMP BAR DETAIL (4 REQUIRED PER POST



CLAMP ASSEMBLY



RAIL CAP



2/13^{2BB69}ABAD4004D3...

Kimley » Horn 421 Fayetteville Street, Suite 600
Raleigh, NC 27601-1772
NC LICENSE #
F-0102

PROJECT NO. 17BP.2.R.78 LENOIR COUNTY STATION: 13+60.00 -L-

MINOR (AXIS

SHEET 2 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

SEMI-ELLIPSE

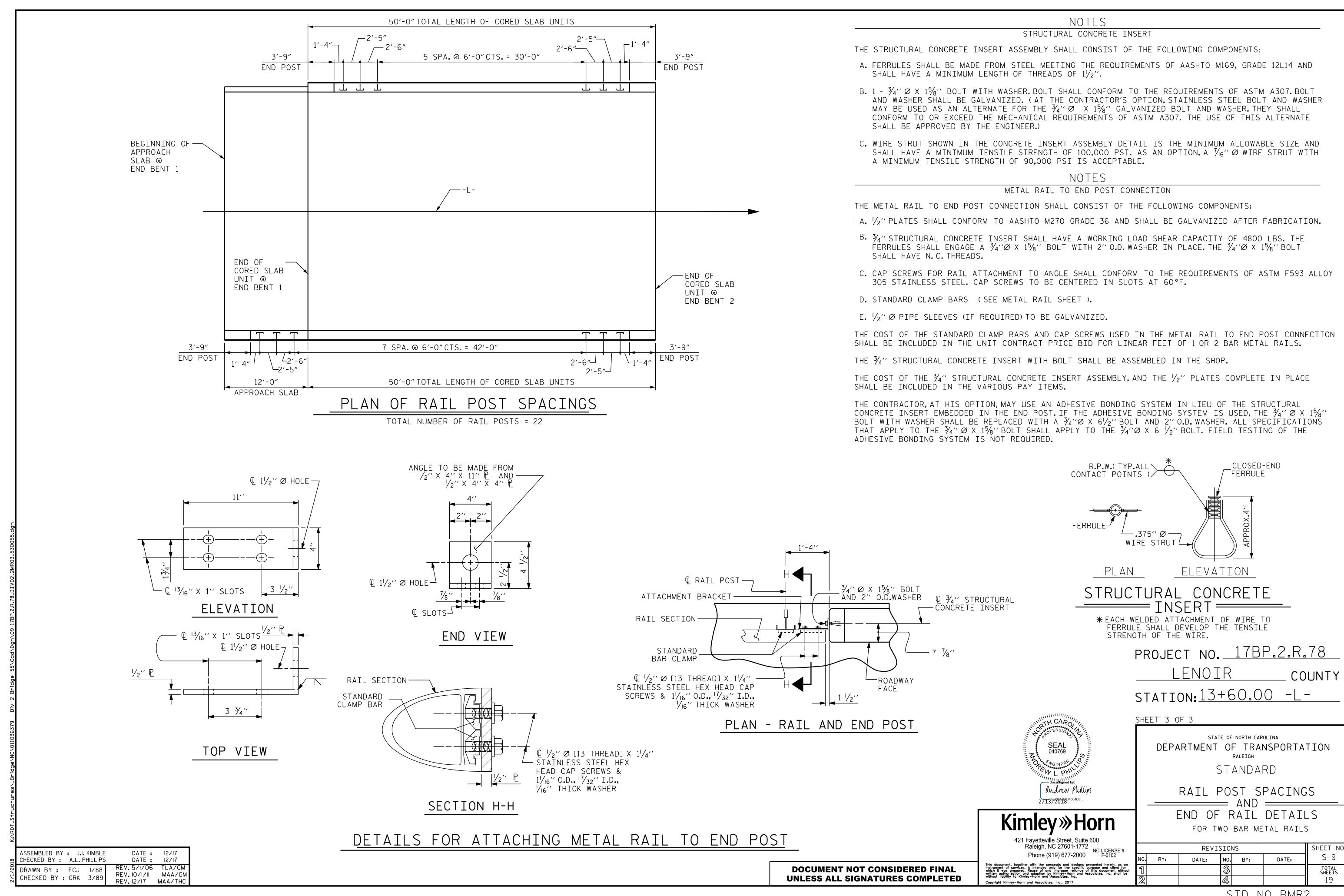
MAJOR AXIS

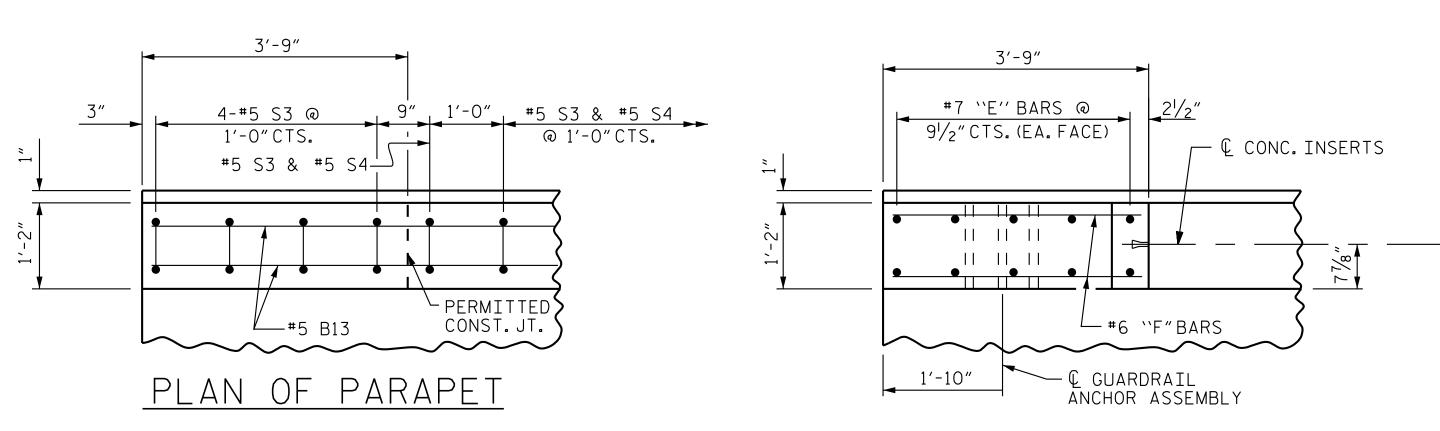
2 BAR METAL RAIL

REVISIONS SHEET NO S-8 NO. BY: DATE: DATE: BY: TOTAL SHEETS

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STD. NO. BMR4

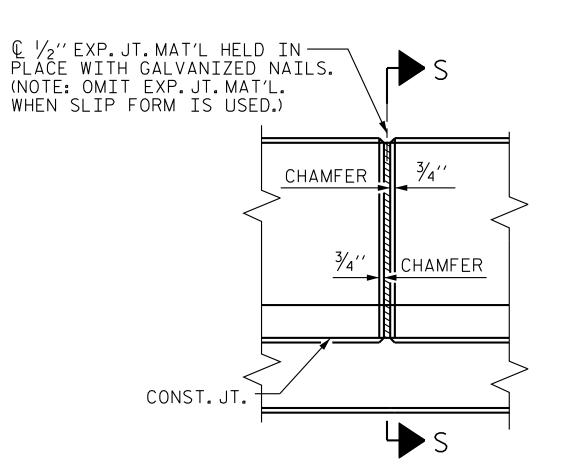




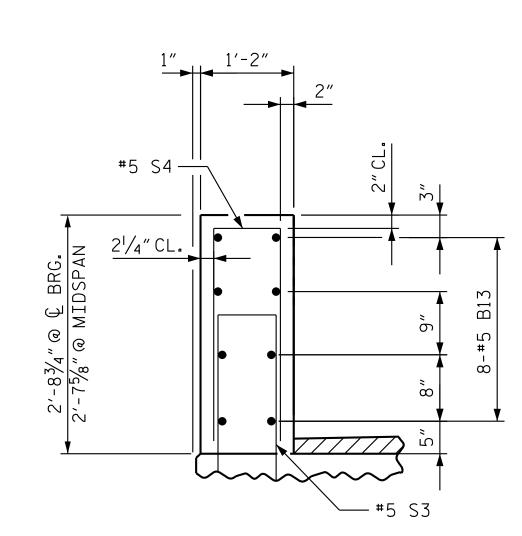
PLAN OF END POST

PLAN OF PARAPET AND END POST FOR TWO BAR RAIL

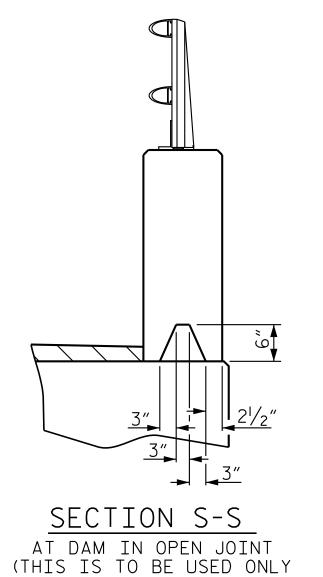
FOR DETAILS ON #5 S3 BARS, SEE SHEETS S-4 THRU S-6.



ELEVATION AT EXPANSION JOINTS



CONCRETE PARAPET SECTION



WHEN SLIP FORM IS USED)

andrew Phillips

421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 NC LICENSE #

2 PARAPETS & 3 END POSTS* BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT *B13 32 5 STR 24'-7" 7 STR 2'-8" 7 STR 3'-2" 39 **∗**E2 | 6 STR 3'-8" 45 51 STR 4'-2" **米**E5 │ 6 │ 7 | STR| 4'-6" 55 6 | STR| 1'-9" 6 STR 2'-11" 26 6 | STR | 3'-3" 29 *F3 6 524 *S4 | 90 | 5 | 1 | 5′-7″ * EPOXY COATED REINFORCING STEEL LBS. CLASS AA CONCRETE CU. YDS. CONCRETE PARAPET LIN.FT. 100.00 BAR TYPE

BILL OF MATERIAL

* FOR DETAILS ON END POST IN SOUTHEAST QUADRANT, SEE SHEETS S-17 THRU S-19.

ALL BAR DIMENSIONS ARE OUT-TO-OUT

PROJECT NO. 17BP.2.R.78 LENOIR COUNTY STATION: 13+60.00 -L-

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

1'-2" X 2'-8¾" CONCRETE PARAPET AND END POSTS

SHEET NO REVISIONS S-10 NO. BY: DATE: DATE: BY: TOTAL SHEETS

3′-9″ #7 "E" BARS @ - PERMITTED 2" CL.TO $9\frac{1}{2}$ CTS. (EA. FACE) CONST.JT. #6 '`F"BAR (TYP.) #7 \`E"BARS -#6 F1 (EA.FACE) - PERMITTED CONST.JT. #6 F2 (EA.FACE) ∕— Û GUARDRAIL ANCHOR ASSEMBLY 2"CL.(TYP.) PERMITTED CONST. JT. CONST.JT.-#5 S3 —

END VIEW

PARAPET AND END POST FOR TWO BAR RAIL

ELEVATION

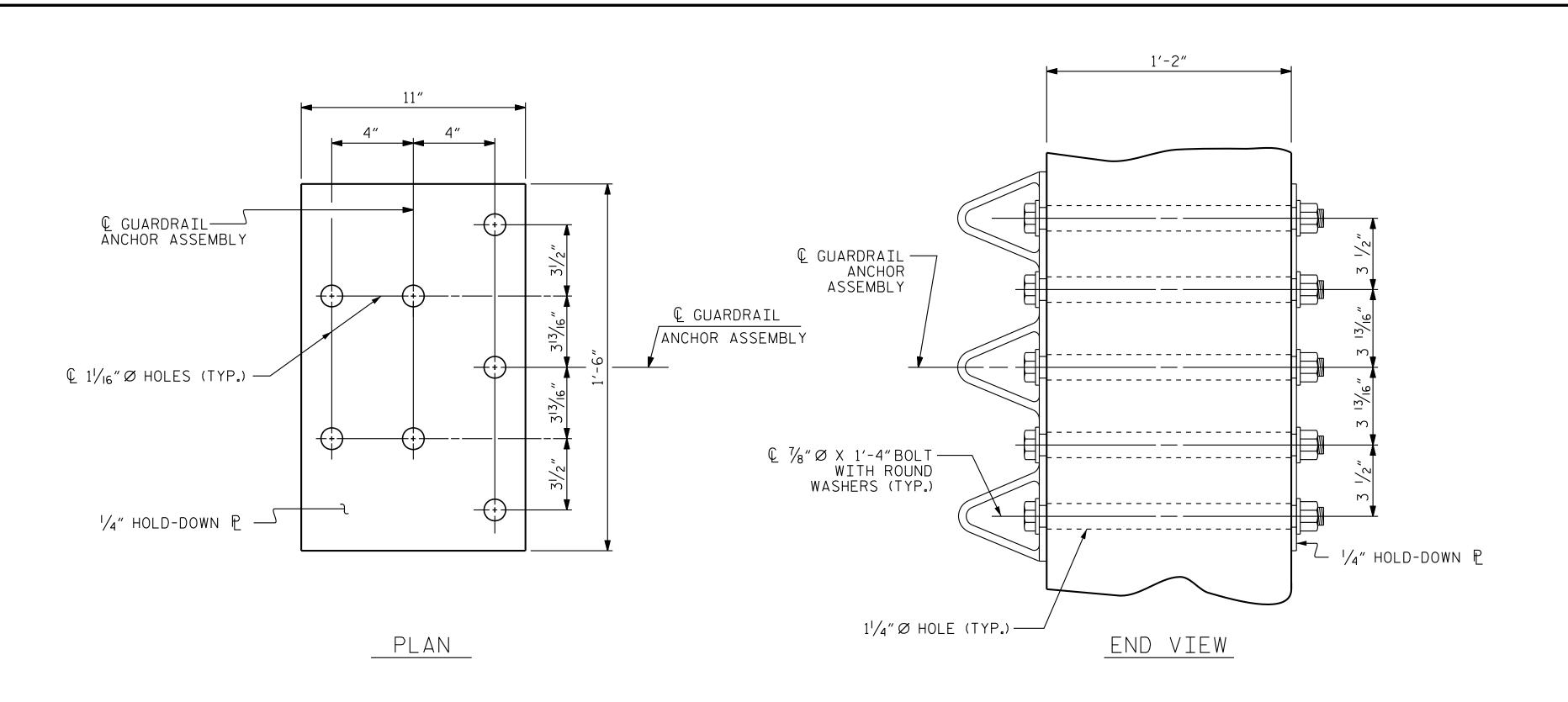
FOR DETAILS ON #5 S3 BARS, SEE SHEETS S-4 THRU S-6.

DATE: 12/17 DRAWN BY: J.I.KIMBL CHECKED BY: S.A. DENNEY DATE: 12/17 DESIGN ENGINEER OF RECORD: A.L. PHILLIPS DATE: 12/17

UNLESS ALL SIGNATURES COMPLETED

DOCUMENT NOT CONSIDERED FINAL

ASSEMBLED BY : J.I. KIMBLE CHECKED BY : A.L. PHILLIPS



GUARDRAIL ANCHOR ASSEMBLY DETAILS

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $1/4^{\prime\prime}$ HOLD DOWN PLATE AND 7 - $1/8^{\prime\prime}$ Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION. THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 7/8' \varnothing GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.

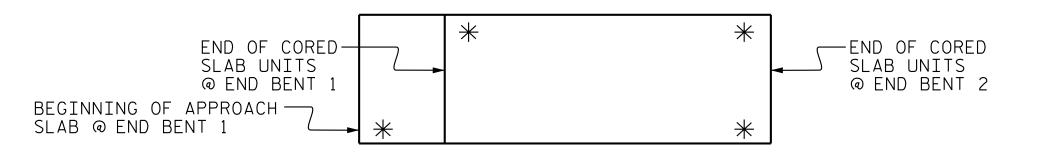
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF THE PARAPET. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLIES WITH BOLTS, NUTS AND WASHERS COMPLETE IN PLACE. SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE END POST TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



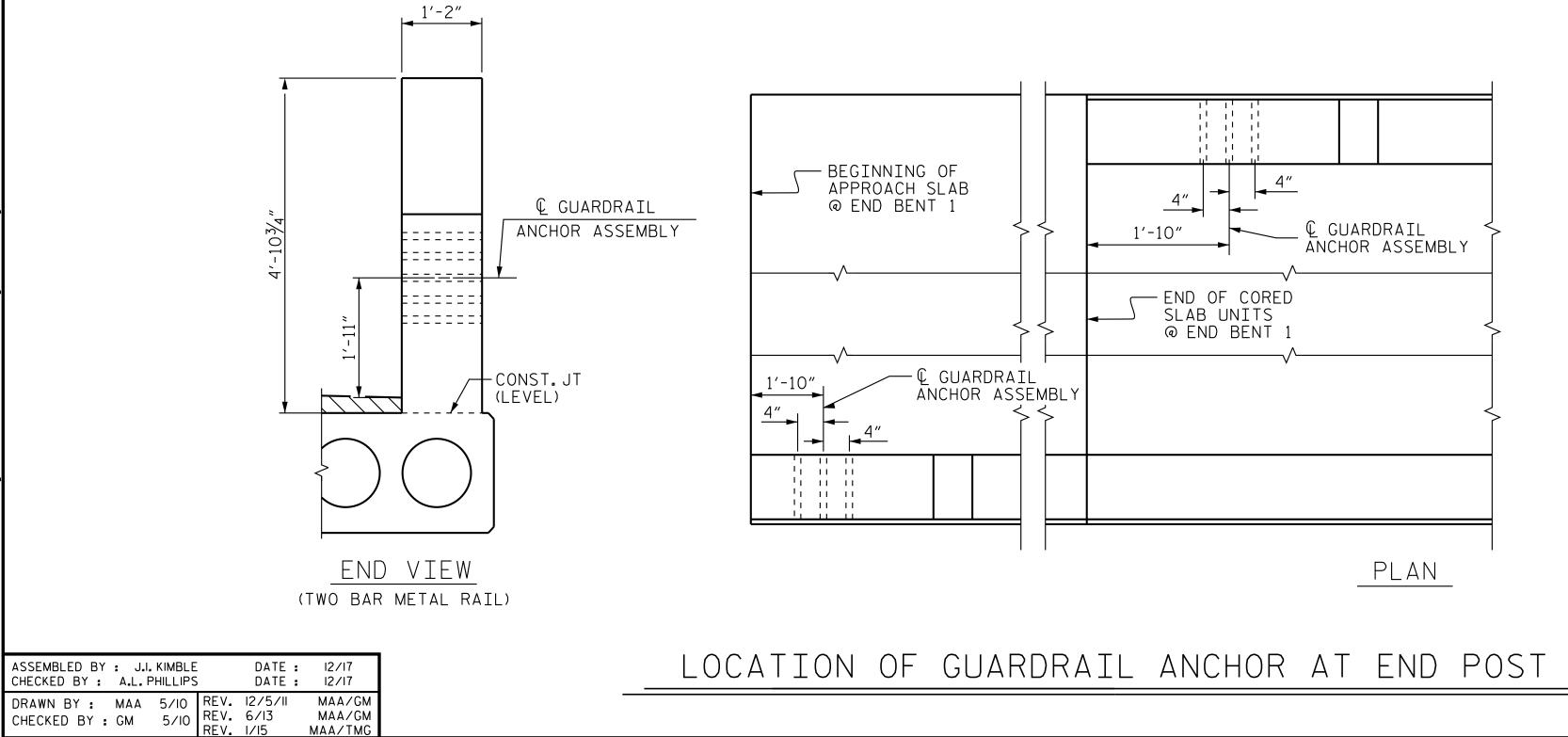
SEAL 040769

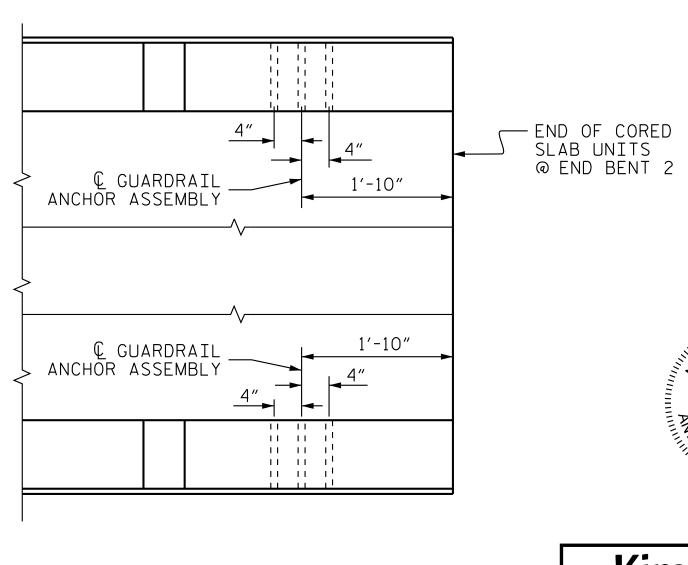
andrew Phillips

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Raleigh, NC 27601-1772
NC LICENSE #
F-0102

SKETCH SHOWING POINTS OF ATTACHMENT

*LOCATION OF GUARDRAIL ATTACHMENT





PROJECT NO. 17BP.2.R.78 <u>ENOIR</u> COUNTY

STATION: 13+60.00 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

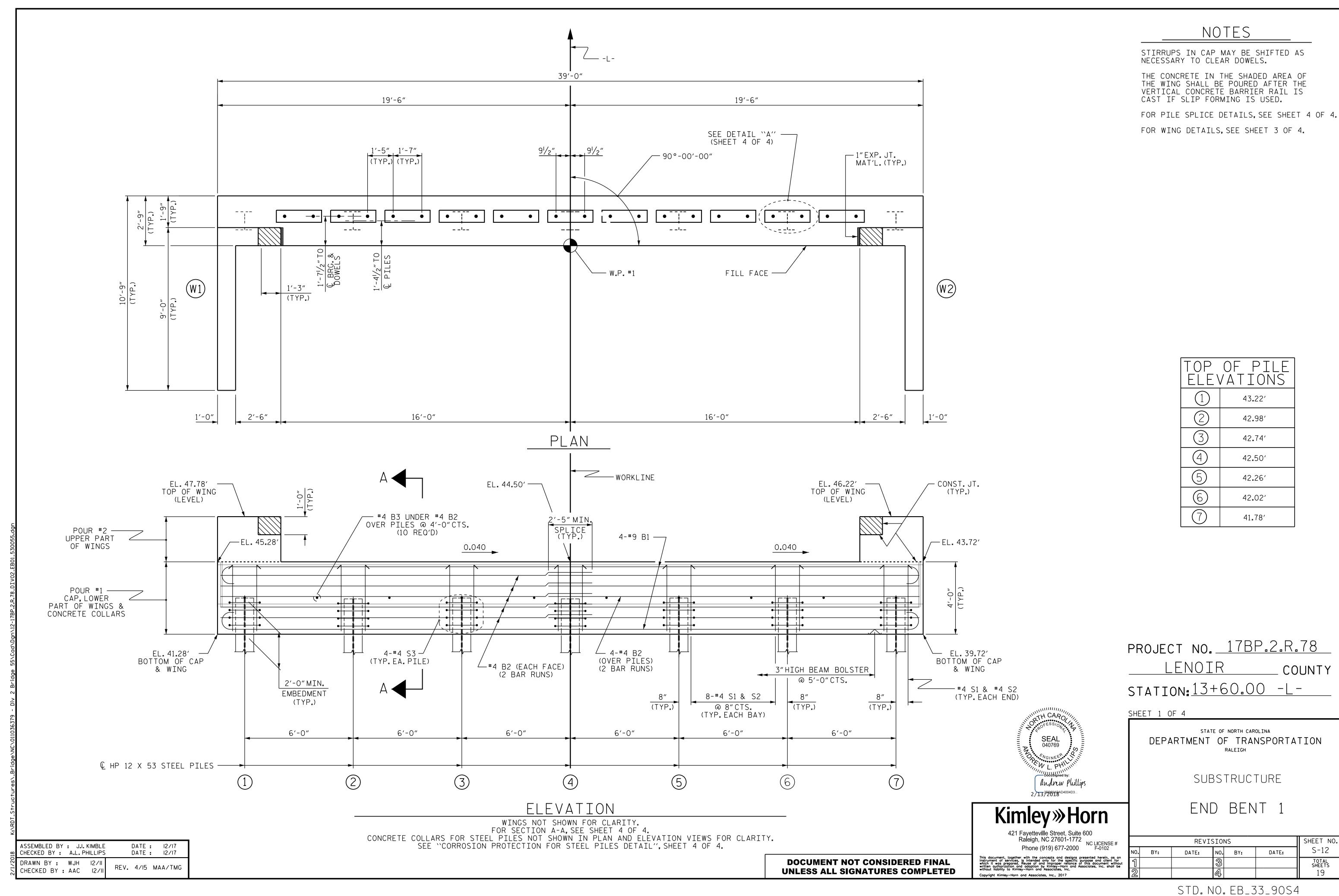
STANDARD

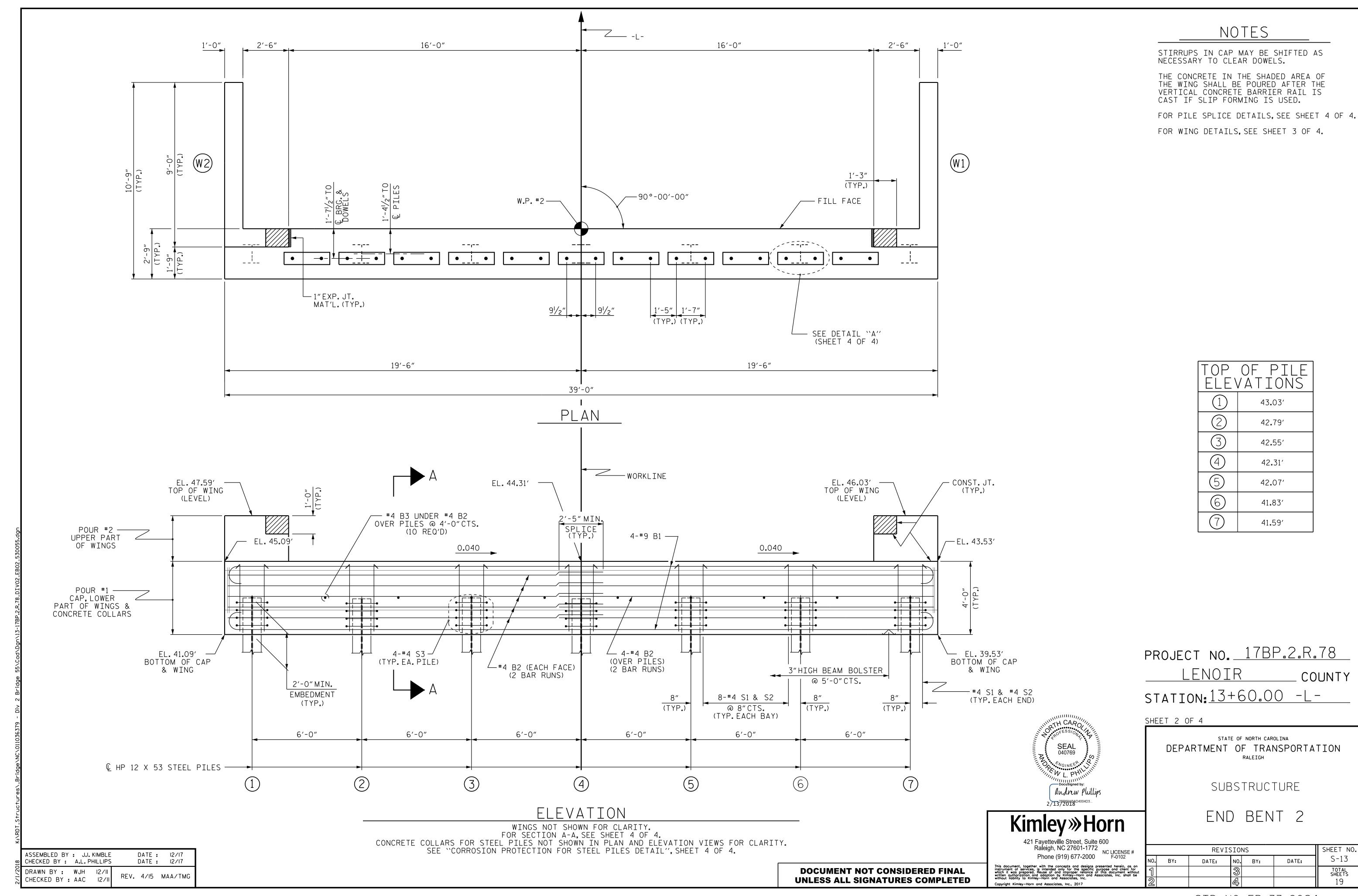
GUARDRAIL ANCHORAGE DETAILS FOR METAL RAILS

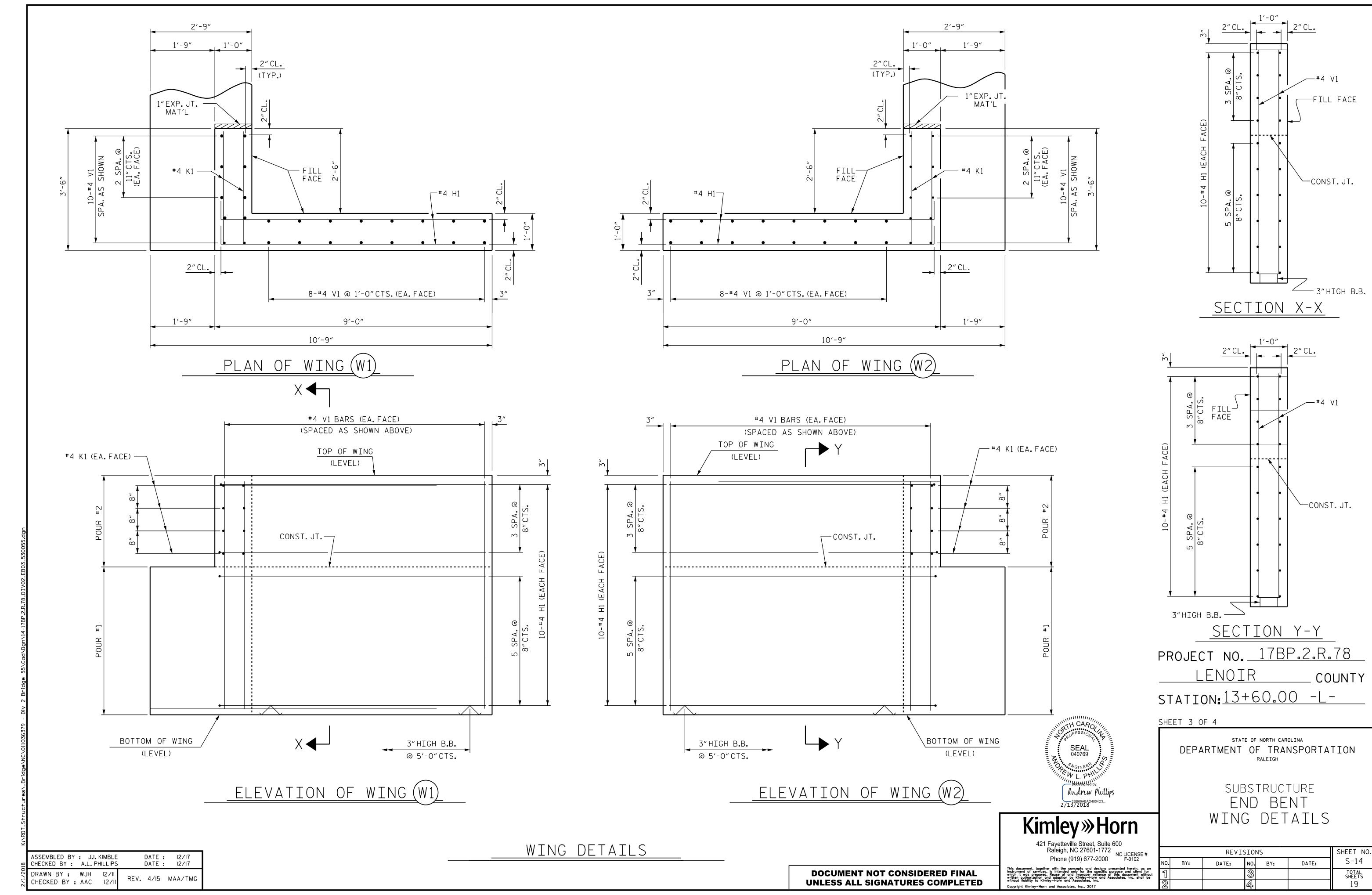
REVISIONS SHEET NO S-11 NO. BY: DATE: BY: DATE: TOTAL SHEETS

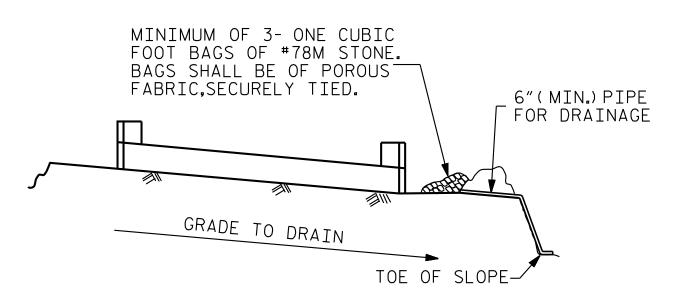
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

STD. NO. GRA3







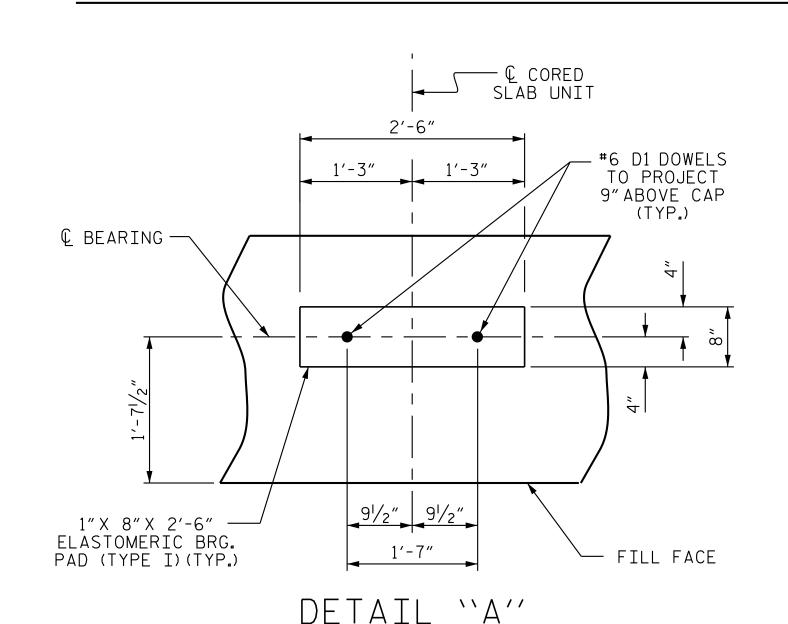


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

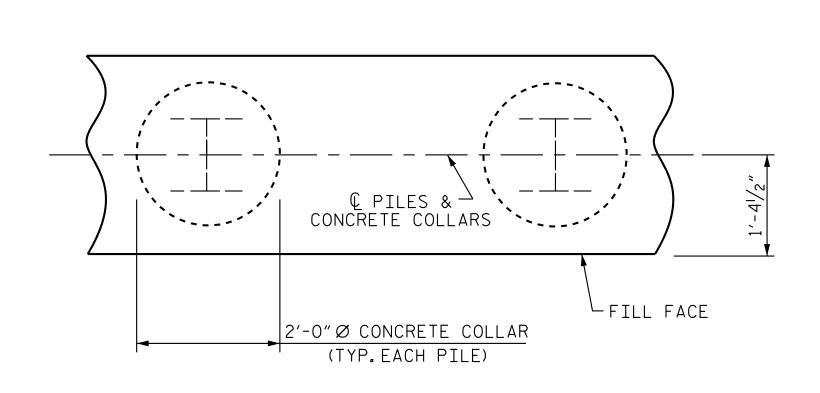
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



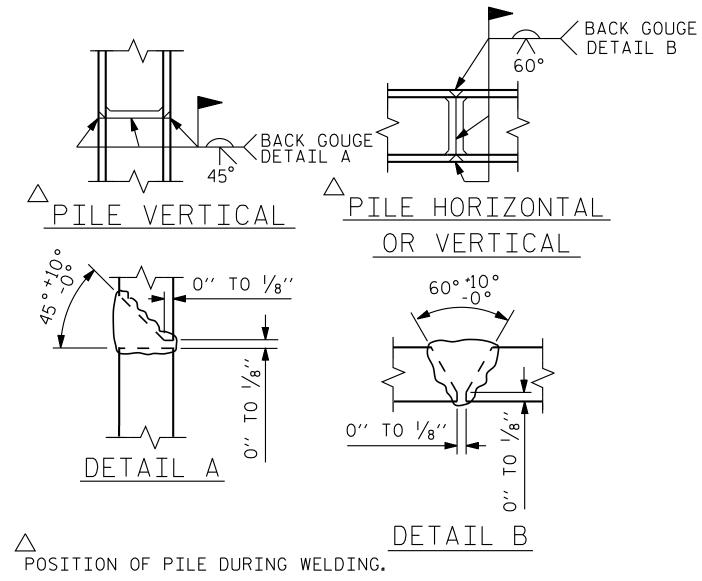
(END BENT 1 SHOWN, END BENT 2 SIMILAR BY ROTATION)



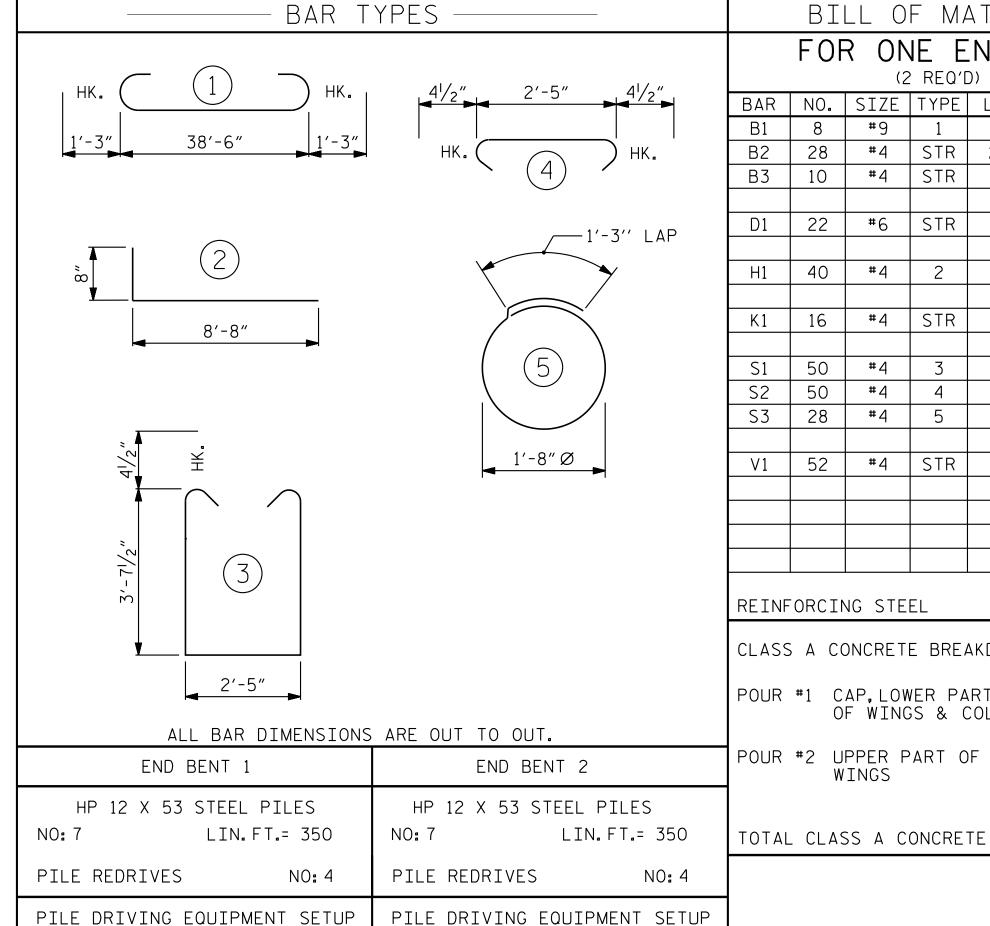
PLAN

CORROSION PROTECTION FOR STEEL PILES DETAIL (END BENT 1 SHOWN, END BENT 2 SIMILAR BY ROTATION)

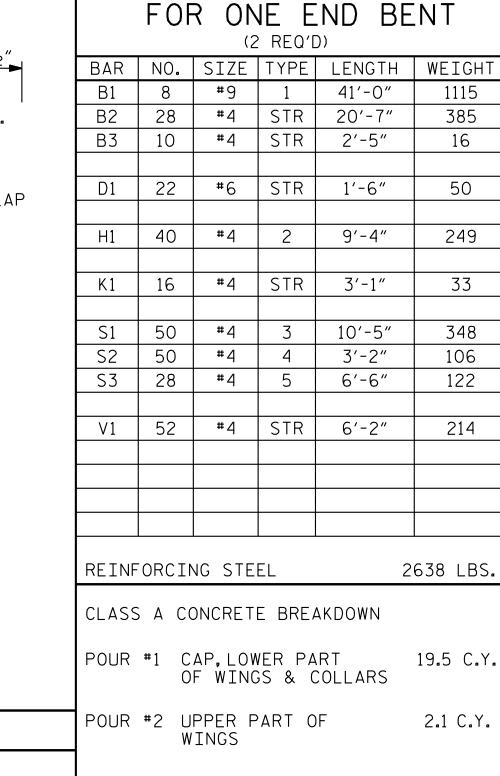
ASSEMBLED BY : J.I. KIMBLE CHECKED BY : A.L. PHILLIPS DATE : 12/17 DATE : 12/17 DRAWN BY : WJH 12/11 REV. 4/I7 MAA/THC CHECKED BY : AAC 12/11



PILE SPLICE DETAILS

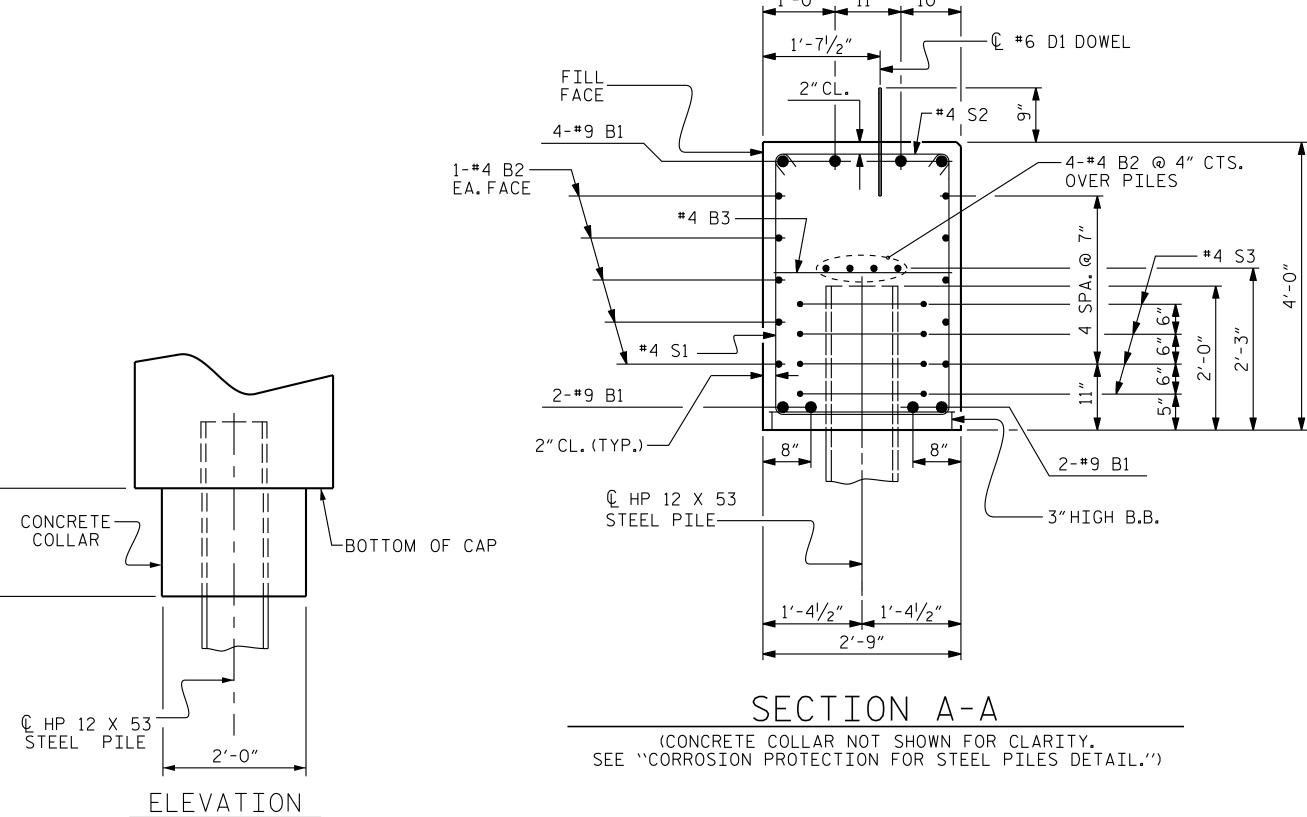


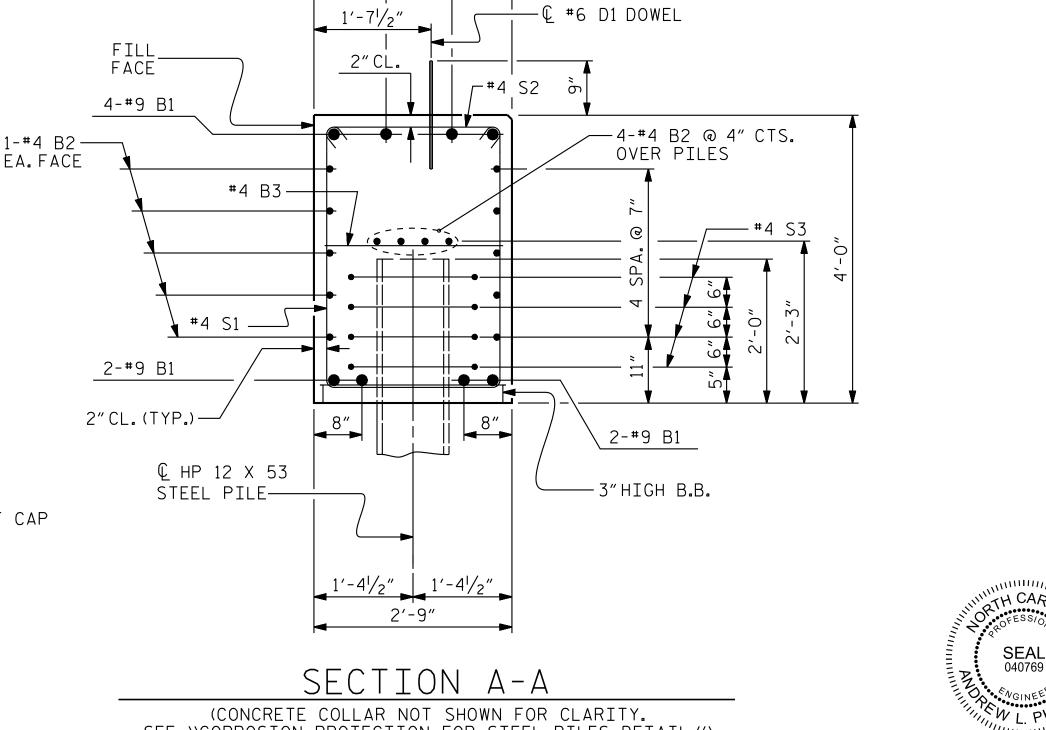
NO: 7



21.6 C.Y.

BILL OF MATERIAL





DOCUMENT NOT CONSIDERED FINAL

UNLESS ALL SIGNATURES COMPLETED

SEAL 040769 andrew Phillips 2/13/2018 D4004D3

PROJECT NO. 17BP.2.R.78 LENOIR COUNTY STATION: 13+60.00 -L-

SHEET 4 OF 4

NO: 7

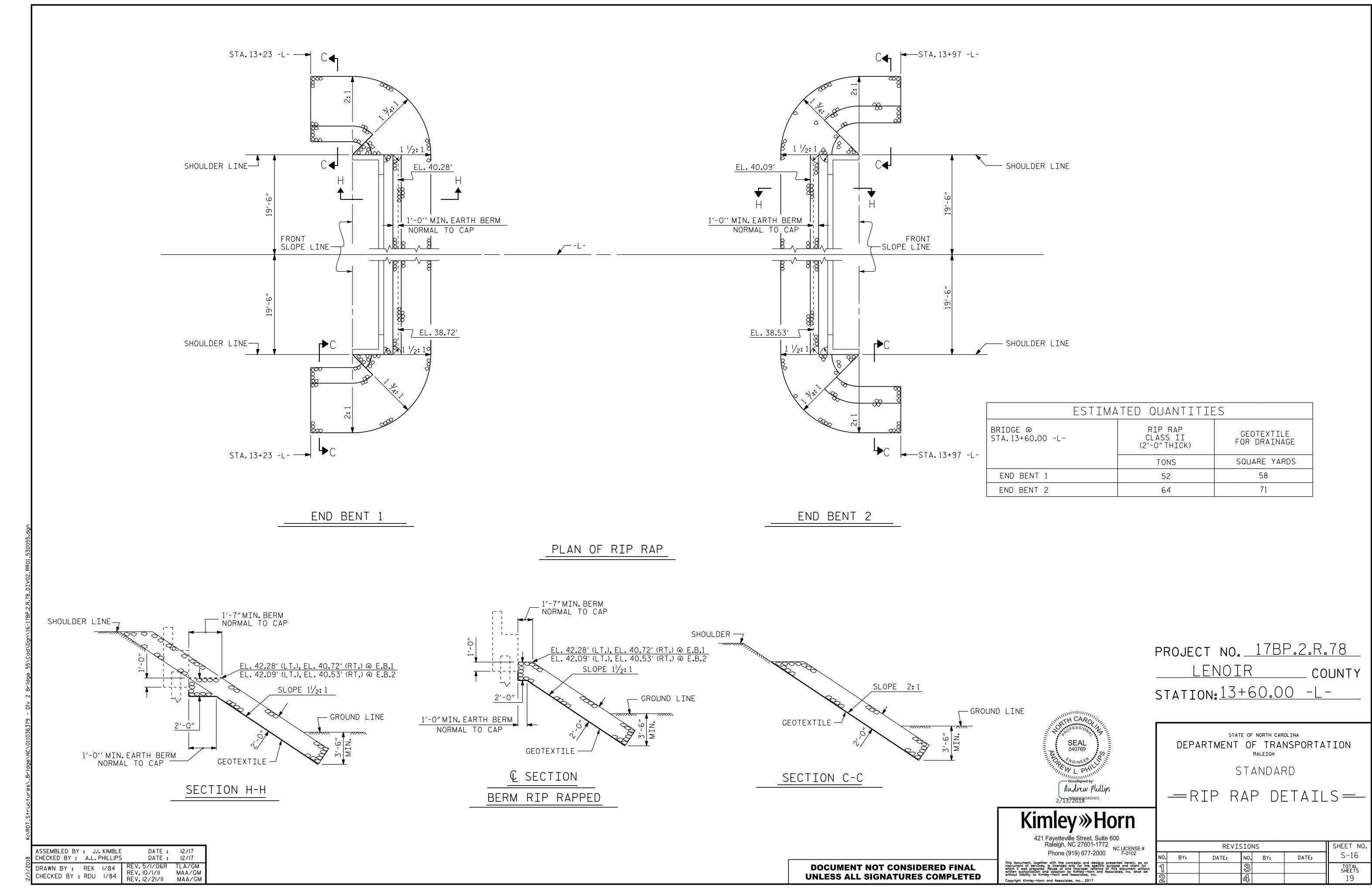
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

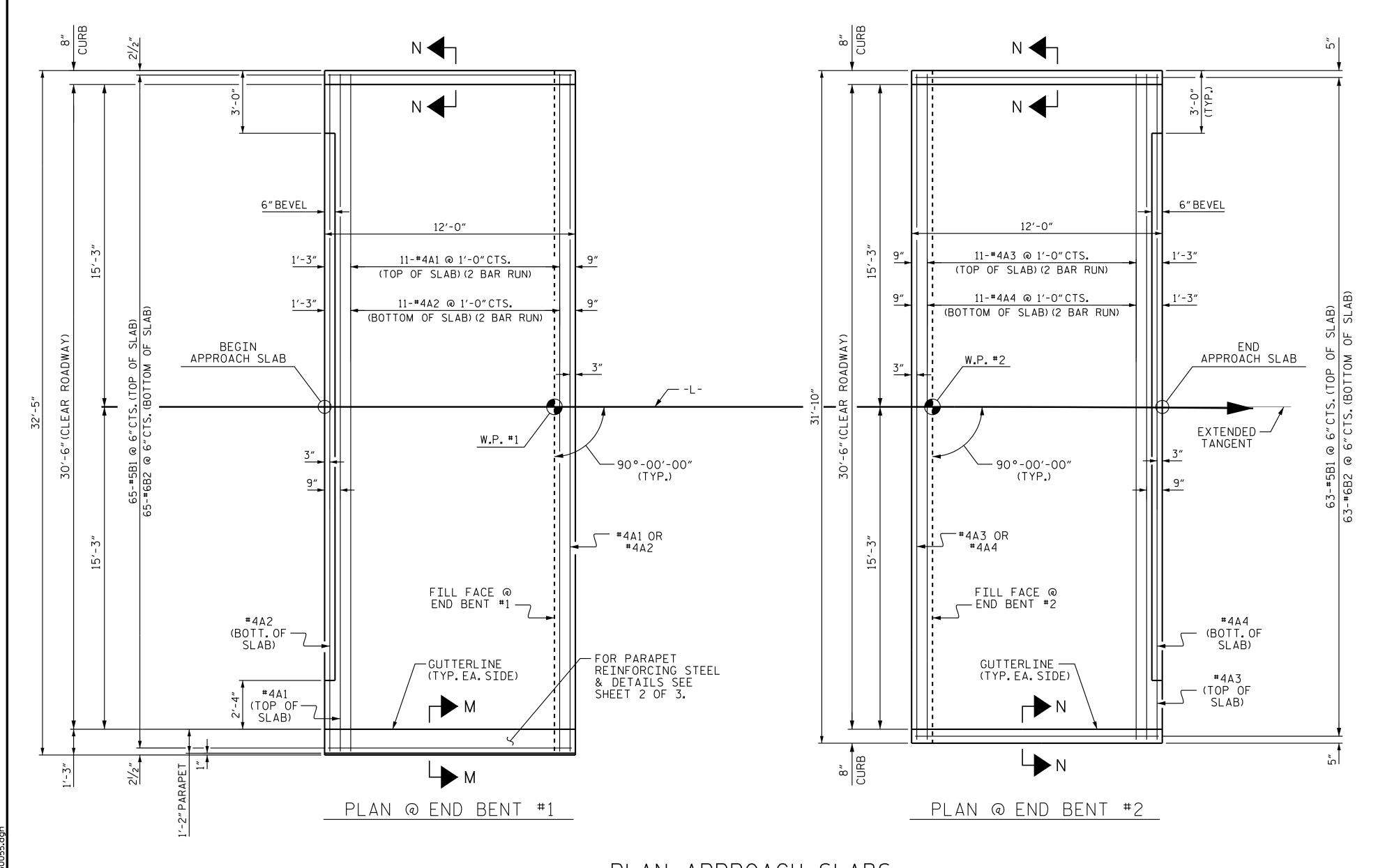
> SUBSTRUCTURE END BENT 1 & 2 DETAILS

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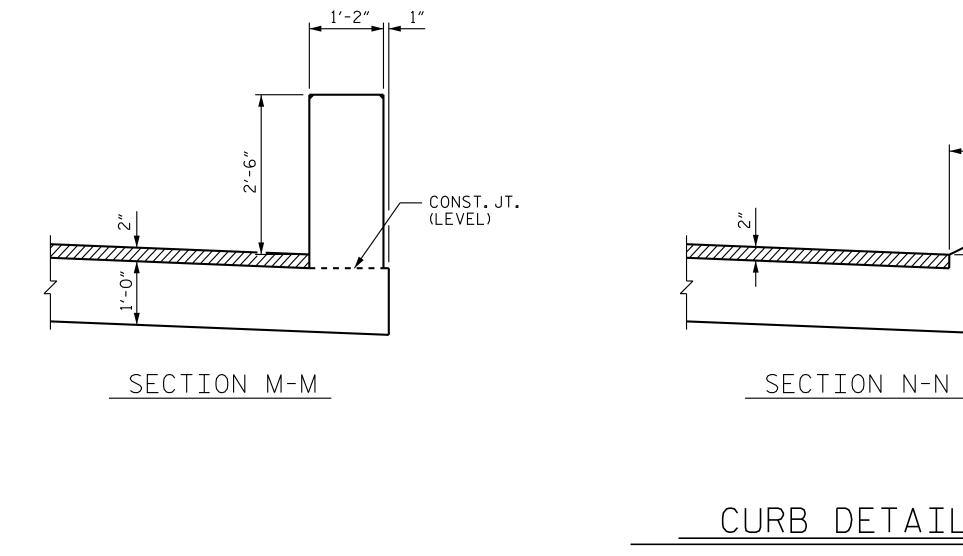
NC LICENSE #

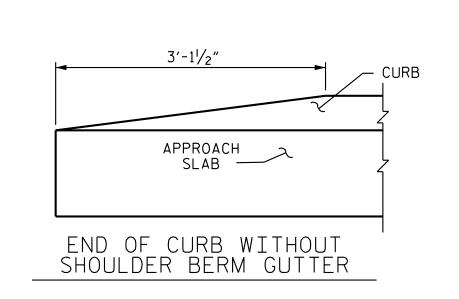
REVISIONS SHEET NO S-15 NO. BY: DATE: DATE: BY: TOTAL SHEETS





PLAN APPROACH SLABS





CURB DETAILS

UNLESS ALL SIGNATURES COMPLETED

NOTES

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 4" Ø DRAINAGE PIPE, AND SELECT MATERIAL BACKFILL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

SELECT MATERIAL BACKFILL (CLASS V OR CLASS VI) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

APPROACH SLAB GROOVING IS NOT REQUIRED.

PROJECT NO. 17BP.2.R.78 LENOIR COUNTY STATION: 13+60.00 -L-

SHEET 1 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE CORED SLABS

SHEET NO REVISIONS S-17 NO. BY: DATE: DATE: BY: TOTAL SHEETS

421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 NC LICENSE #

SEAL 040769

Docusigned by:

Andrew Phillips

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ASSEMBLED BY : J.I. KIMBLE CHECKED BY : A.L. PHILLIPS

DRAWN BY :SHS/MAA 5-09
CHECKED BY : BCH 5-09
REV. 12-17

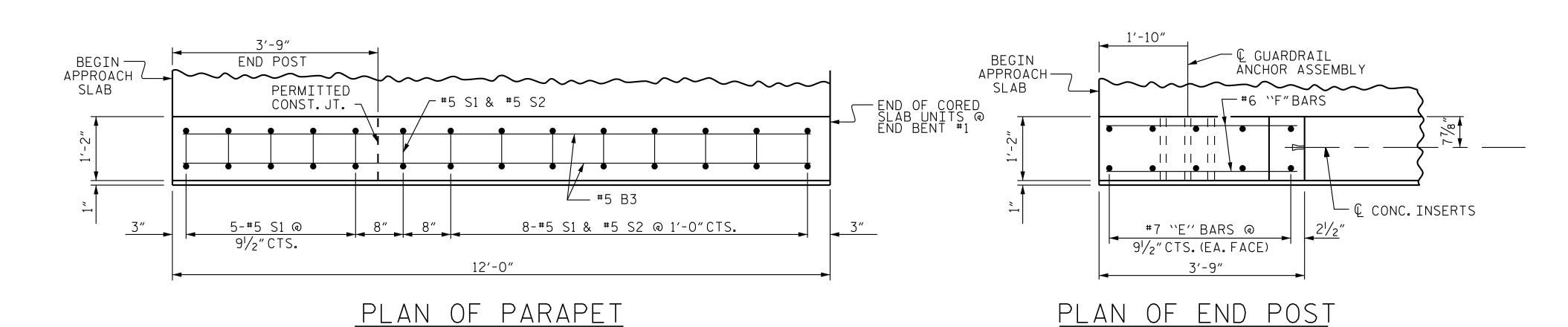
DATE : 12/17 DATE : 12/17

MAA/THC

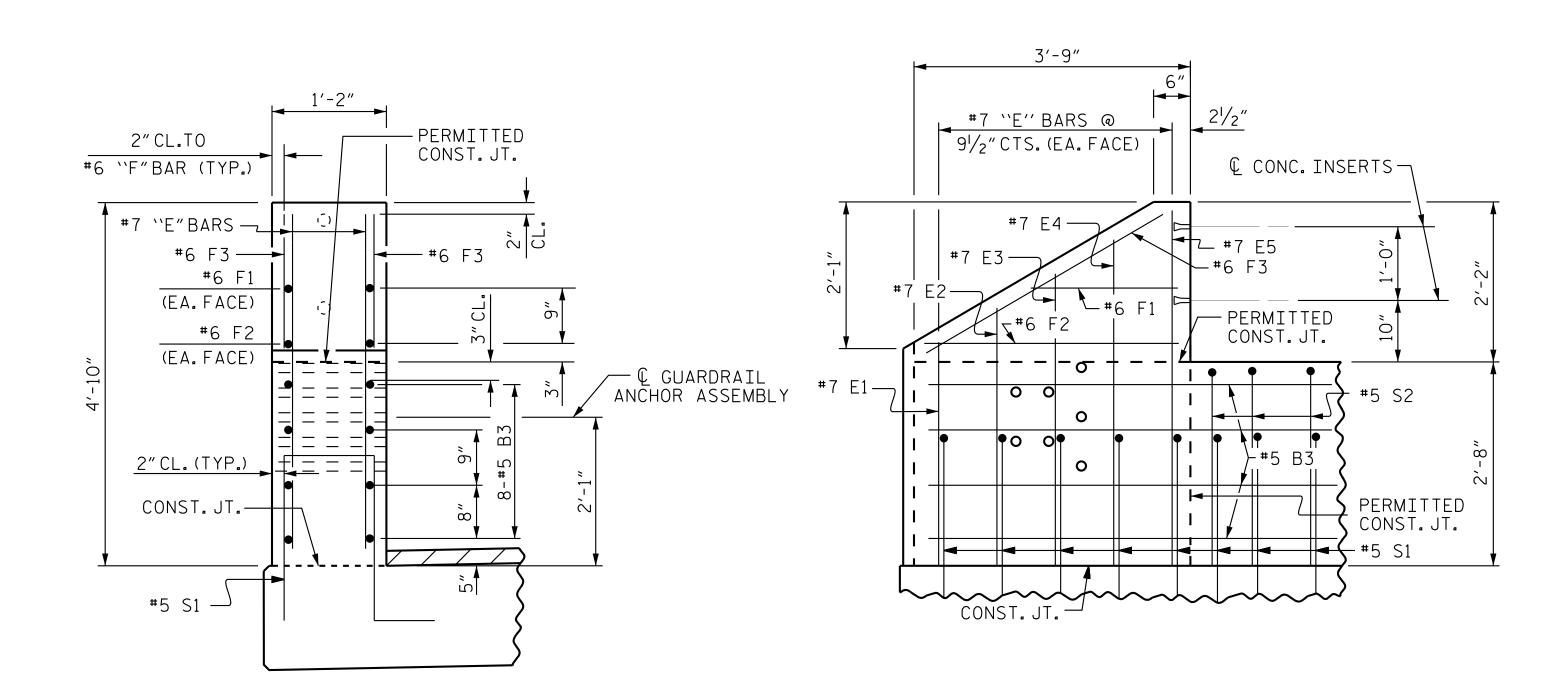
NOTES

FOR EXPANSION JOINT DETAILS, SEE SHEET S-10.

FOR CONCRETE PARAPET AND END POST BILL OF MATERIALS AND OTHER DETAILS, SEE SHEET S-19.



<u>Plan of parapet and end post for two bar rail</u>



END VIEW

ELEVATION

PARAPET AND END POST FOR TWO BAR RAIL

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SHEET 2 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SLAB DETAILS FOR FLEXIBLE PAVEMENT WITH CONCRETE PARAPET

REVISIONS SHEET NO S-18 NO. BY: DATE: DATE: BY: TOTAL SHEETS

UNLESS ALL SIGNATURES COMPLETED

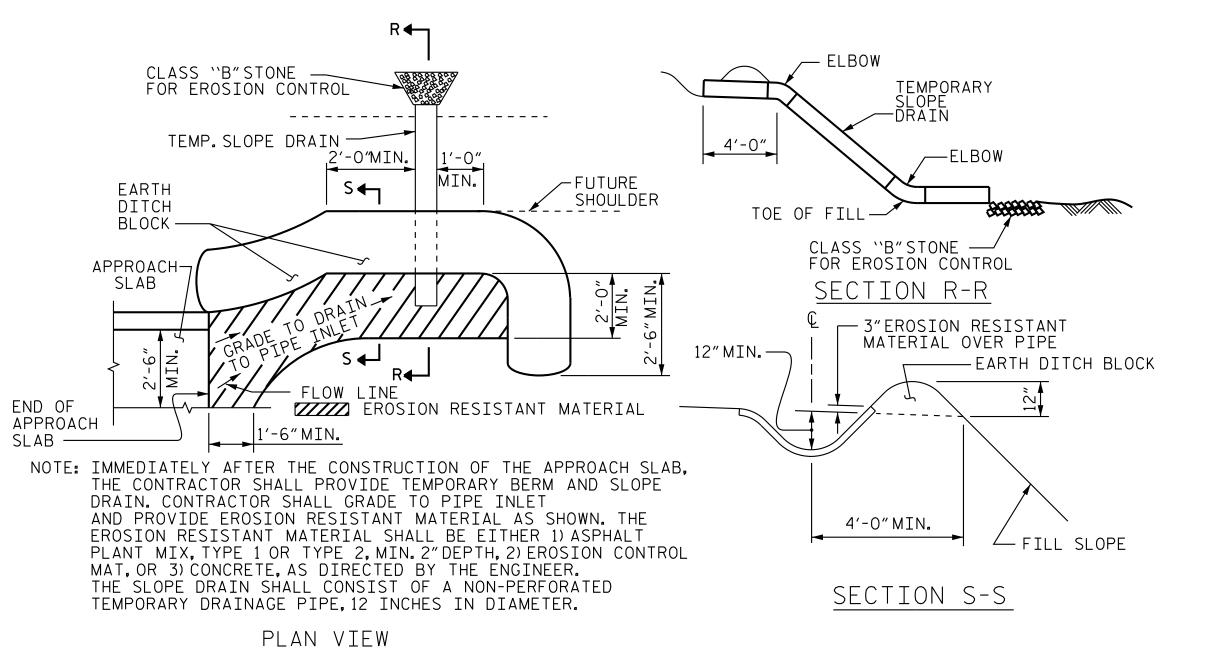
DOCUMENT NOT CONSIDERED FINAL

DATE: 12/17 DRAWN BY: <u>J.I.KIMBLE</u> CHECKED BY: S.A. DENNEY DATE: 12/17 DESIGN ENGINEER OF RECORD: A.L. PHILLIPS DATE: 12/17

CONCRETE PARAPET SECTION

#5 S2-

2"CL.



TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)

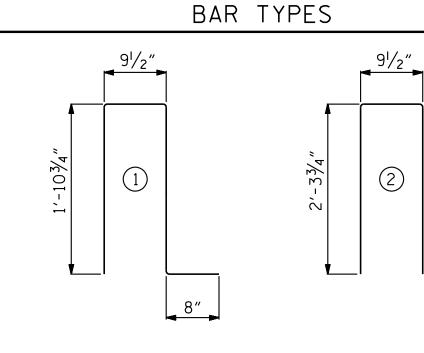
BRIDGE DECK

CAP FLOW LINE ONLY WITH
EROSION RESISTANT MATERIAL

BACKFILL EXCAVATION HOLE
AND GRADE TO DRAIN

NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL



ALL BAR DIMENSIONS ARE OUT TO OUT

SPLICE LENGTHS							
BAR SIZE	EPOXY COATED	UNCOATED					
#4	2'-0"	1'-9"					
#5	2'-6"	2'-2"					
#6	3′-10″	2'-7"					

∗ F1	2	#6	STR	1'-9"	5					
* F2	2	#6	STR	2'-11"	9					
∗ F3	2	#6	STR	3′-3″	10					
* S1	14	#5	1	5′-3″	77					
* S2	9	#5	2	5′-5″	51					
REINF	ORCIN	G STEE	L	LBS.	1433					
	XY CO NFORC	ATED Ing st	LBS.	1377						
		CLASS	AA CC	NCRETE						
POUR	POUR #1 - SLAB & CURB				18.4					
POUR #2 - PARAPET				C.Y.	1.6					
TOTAL	-		C.Y.	20.0						
CONCRETE PARAPET				LIN.FT.	12.0					
APPROACH SLAB AT EB #2										
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT					
* ∆3	26	#4	STR	16′-9″	291					
Α4	26	#4	STR	16′-8″	289					
∗ B1	63	#5	STR	11'-2"	734					
B2	63	#6	STR	11'-8"	1104					
REINF	ORCIN	G STEE	LBS.	1393						
	XY CO NFORC	ATED ING ST	LBS.	1025						
CLASS	AA C	ONCRET	C.Y.	18.2						

BILL OF MATERIAL

APPROACH SLAB AT EB #1

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

#4 | STR |

#5 | STR |

#6 STR

#5 | STR |

#7 | STR |

#7 | STR |

#7 | STR

#7 STR

#7 | STR |

#4 | STR | 17′-1″

16′-11″

11'-2"

11′-8″

11'-7"

2′-8″

3′-2″

3′-8″

4′-2″

4′-6″

297

294

757

1139

97

13

17

18

* A1 | 26 |

∗ B1

* B3

∗ E1

∗ E2

∗ E3

* E4 |

* E5 | 2

A2 | 26 |

65 |

65

PROJECT NO. 17BP.2.R.78

LENOIR COUNTY

STATION: 13+60.00 -L-

SHEET 3 OF 3

andrew Phillips

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE CORED SLAB UNIT (SUB-REGIONAL TIER)

REVISIONS

BY: DATE: NO. BY: DATE: S-19

TOTAL SHEETS
19

— 51/4" CONTINUOUS HÍGH CHAIR UPPER (CHCU) -PROPOSED @ 3'-0"CTS.ACROSS SLAB ASPHALT PAVEMENT /-- #5B1 /— #4``A″ CORED SLAB 🗾 ROADWAY— [†]2 :1 SLOPE -#6B2 — —11/2"BACKER ROD - 2 LAYERS OF 30 LB. ROOFING FELT TO PREVENT BOND -SELECT APPROVED WIRE BAR -MATERIAL SUPPORTS @ 3'-0"CTS. APPROXIMATE-1: 1 SLOPE (TO BE DETERMINED BY THE CONTRACTOR) GEOTEXTILE 4"Ø PERFORATED -SCHEDULE 40 PVC PIPE † NORMAL TO END BENT 3′-0″ SECTION THRU SLAB

(TYPE II - MODIFIED APPROACH FILL)

Kinley >>> Horn

421 Fayetteville Street, Suite 600
Raleigh, NC 27601-1772
Phone (919) 677-2000
F-0102

This document, together with the concepts and designs presented herein, as a

ASSEMBLED BY: J.I. KIMBLE DATE: 12/17
CHECKED BY: A.L. PHILLIPS DATE: 12/17

DRAWN BY: SHS/MAA 5-09
CHECKED BY: BCH 5-09

REV. 12-17

MAA/THC

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STANDARD NOTES

DESIGN DATA:

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N.C. DEPARTMENT OF TRANSPORTATION.

EQUIVALENT FLUID PRESSURE OF EARTH - - - - 30 LBS. PER CU. FT.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4 WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2 RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4 FINISHING TOOL UNLESS OTHERWISE REOUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4 RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ % SHEAR STUDS FOR THE $\frac{7}{4}$ % STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ % STUDS FOR 4 - $\frac{7}{4}$ % STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ % STUDS ALONG THE BEAM, AS SHOWN FOR $\frac{7}{4}$ % STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ % STUDS FOR 4 - $\frac{7}{4}$ % STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 1/6 TIN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLITZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

JANUARY, 1990